

GOVERNO DO ESTADO



**GOVERNO DO ESTADO DO CEARÁ**  
**SECRETARIA DOS RECURSOS HÍDRICOS - SRH**  
**COMPANHIA DE GESTÃO DOS RECURSOS HÍDRICOS COGERH**  
**PROJETO DE DESENVOLVIMENTO URBANO E GESTÃO DOS**  
**RECURSOS HÍDRICOS PROURB / CE**

**PROJETO EXECUTIVO DA BARRAGEM ARACOIABA**  
**E ADUTORAS DE ARACOIABA E BATURITÉ**

**RELATÓRIO DE CONCEPÇÃO**  
**VOLUME 3 MEMÓRIA DE CÁLCULO**

**RHAR - 971210 - RE**

**CONSÓRCIO GEODINÂMICA COBA**

**FORTALEZA- CE**  
**DEZEMBRO DE 1997**

GOVERNO DO ESTADO



**CEARÁ**  
AVANÇANDO NAS MUDANÇAS

**GOVERNO DO ESTADO DO CEARÁ**  
**SECRETARIA DOS RECURSOS HÍDRICOS**  
**COMPANHIA DE GESTÃO DOS RECURSOS HÍDRICOS - COGERH**  
**PROJETO DE DESENVOLVIMENTO URBANO E GESTÃO DOS RECURSOS HÍDRICOS**  
**PROURB/CE**

**PROJETO EXECUTIVO DA BARRAGEM ARACOIABA**  
**E ADUTORAS ARACOIABA E BATURITÉ**

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Projeto Nº 0217/03/A  
Volume \_\_\_\_\_  
Qtd A4 \_\_\_\_\_ Qtd A3 \_\_\_\_\_  
Qtd A2 \_\_\_\_\_ Qtd A1 \_\_\_\_\_  
Qtd A0 \_\_\_\_\_ Outros \_\_\_\_\_

**RELATÓRIO DE CONCEPÇÃO**  
**MEMÓRIAS DE CÁLCULO**

**VOLUME 3**

**RHAR-971210-RE**



**CONSÓRCIO GEODINÂMICA COBA**

**FORTALEZA**  
**DEZEMBRO/97**

0217/03/A

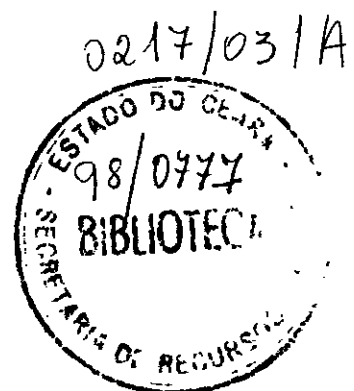
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## 1 - APRESENTAÇÃO

Neste volume estão apresentadas as memórias de cálculo relativas aos estudos do projeto executivo para a barragem do açude de Aracoiaba

Os estudos correspondem à

- Estudos Hidráulicos
- Cálculos dos quantitativos da barragem de terra
- Cálculos dos quantitativos das estruturas hidráulicas
- Análise de estabilidade dos taludes da barragem
- Estudos de Otimização



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## 2 - ESTUDOS HIDRÁULICOS

A seguir são apresentadas as memórias de cálculo relativas aos estudos hidráulicos

- Anexo 2 1 - Vertedouro Soleira Livre - cálculos da geração do hidrograma decamilenar para a chuva com 24 h de duração
- Anexo 2 2 - Vertedouro Soleira Livre - cálculos do amortecimento da descarga de projeto para T=1 000 anos
- Anexo 2 3 - Vertedouro Soleira Livre - cálculos do amortecimento da descarga de projeto para a cheia de verificação de T=10 000 anos
- Anexo 2 4 - Vertedouro tipo Tulipa - cálculos de laminação para cheia de projeto de T=1 000 anos
- Anexo 2 5 - Vertedouro tipo Tulipa - cálculos de laminação para cheia de projeto de T=10 000 anos
- Anexo 2 6 - Desvio de 1º Fase - cálculos de verificação da brecha
- Anexo 2 7 - Galeria de Desvio - Dimensionamento para T=50 anos

## ANEXO 2.1

BACIA DO RIO ARACÓIARA  
 VERIFICAÇÃO DA INCHENTE DECAMILÉNAR T=10 000 anos  
 (chuva  $\phi=24$ horas  $\Rightarrow$  P=254.3 mm)

ARQUIVO T2410000 I      DATA 21/09/97

AREA DE DRENAGEM = 584 050 km<sup>2</sup>  
 COMP TALVEGUE = 53.1 km  
 DIFERENÇA DE COTAS = 765.0 m

CURVE NUMBER      73  
 TA      290.8 min  
 TB      775.5  
 TC      434.7  
 DEL      60.0

### REGIÕES DEFINIDAS

REGIÕES	TIPO DE SOLO	COBERTURA	CONDICOES	PORCENTAGEM
1	B	10	P	100.00

TEMPO (min)	PRECIPITAÇÃO (mm)		RUN-OFF (mm)		VOLUME DE RUN-OFF (m <sup>3</sup> )	PICO DA KIDROG (m <sup>3</sup> /S)
	SIMPLS	ACUMULADA	SIMPLS	ACUMULADO		
60.0	18.82	18.82	00	00	5.96	00
120.0	19.07	37.89	3.23	3.23	1884964.00	81.03
180.0	29.23	66.12	12.63	15.86	7376323.00	317.07
240.0	33.57	99.69	21.58	37.43	12601280.00	541.67
300.0	31.53	131.22	23.82	61.25	13911000.00	597.97
360.0	22.12	153.34	17.98	79.23	10501240.00	451.40
420.0	22.12	175.46	18.71	97.94	10928070.00	469.75
480.0	11.95	187.41	10.35	108.29	6043304.00	259.77
540.0	9.41	196.82	8.25	116.54	4816675.00	207.05
600.0	8.65	205.47	7.65	124.19	4467837.00	192.05
660.0	6.61	212.08	5.89	130.07	3437875.00	147.78
720.0	6.61	218.69	5.92	135.99	3456880.00	148.60
780.0	4.07	222.76	3.66	139.65	2137438.00	91.88
840.0	4.07	226.83	3.67	143.32	2143899.00	92.16
900.0	4.07	230.90	3.68	147.00	2150093.00	92.42
960.0	4.07	234.97	3.69	150.69	2156055.00	92.68
1020.0	4.07	239.04	3.70	154.40	2161785.00	92.93
1080.0	4.07	243.11	3.71	158.11	2167293.00	93.16
1140.0	4.07	247.18	3.72	161.83	2172587.00	93.39
1200.0	2.03	249.21	1.86	163.68	1085540.00	46.66
1260.0	1.27	250.48	1.16	164.85	679763.80	29.22
1320.0	1.27	251.75	1.16	166.01	680253.90	29.24
1380.0	1.27	253.02	1.17	167.18	680726.30	29.26
1440.0	1.27	254.29	1.17	168.35	681189.60	29.28

## ANEXO 2.1 (CONTINUAÇÃO)

BACIA DO RIO ARACOIARA  
 VERIFICAÇÃO DA ESCURRENTA DECAIMILAR T=10 000 anos  
 (chuva d=24horas => P=254,3 mm)

### HIDROGRAFA SINTETICA COMPOSTA

TEMPO (min)	VAZAO COMPOSTA (m <sup>3</sup> /seg)	(Q/QFICO)
60 00	00	000
120 00	16 72	007
180 00	98 86	044
240 00	292 76	130
300 00	610 05	270
360 00	1016 36	450
420 00	1480 90	656
480 00	1982 99	834
540 00	2146 12	950
600 00	2258 90	1 000
660 00	2252 22	997
720 00	2131 75	944
780 00	1947 16	862
840 00	1714 74	759
900 00	1452 48	643
960 00	1201 87	532
1020 00	991 84	439
1080 00	843 34	373
1140 00	739 73	327
1200 00	671 40	297
1260 00	610 16	270
1320 00	549 76	243
1380 00	487 98	216
1440 00	422 09	187
1500 00	359 54	159
1560 00	298 72	132
1620 00	239 65	106
1680 00	182 37	081
1740 00	128 38	057
1800 00	85 90	038
1860 00	54 95	024
1920 00	35 13	016
1980 00	20 92	009
2040 00	10 32	005
2100 00	3 35	001

000006

## ANEXO 2.2

**BARRAGEM DO RIO ARACOIABA - Dimensionamento do vertedouro**  
 Soleira livre L=20 m. Descarga de projeto T=1 000 anos, chuva d= 8 horas

**HIDROGRAMA PADRAO (Q/QPICO) AFLUENTE**

000	036	095	212	306	496	664	802	908	981	1 000	970	861	745
620	452	299	240	146	080	036	000						

**DADOS DA CURVA COTA-VOLUME (10\*\*6 M3)**

65 000	000	70 000	1 090	75 000	8 060	80 000	26 040
85 000	57 020	90 000	104 350	95 000	171 380	100 000	262 780

**DADOS DE COTA-VAZAO (M3/S)**

95 000	000	95 500	14 100	96 000	40 000	96 500	73 500
97 000	113 100	97 500	158 100	98 000	207 900	98 500	261 900
99 000	320 000						

.....

**BARRAGEM DO RIO ARACOIABA - Dimensionamento do vertedouro**  
 Soleira livre L=20 m Descarga de projeto T=1 000 anos chuva d= 8 horas

.....

TEMPO (HORAS)	VAZAO		AFLUENTE		VOLUME		R A (M)	DESCARGA (M3/S)	GRAFICO DE VAZUES (M3/S)					(A)=AFLUENTE TOTAL (D)=DEFLUENTE	
	AFLUENTE (M3/S)	INFORMAD (M3/S)	TOTAL (M3/S)	RESERVAT (M3/S)	RESERVAT (M3/S)	0			300	600	900	1200	1500		
00	00	00	00	00	171 38	95 00	00	00							
1	49 36	00	49 36	171 47	95 00	14	14	A							
2	130 24	00	130 24	171 79	95 02	63	63	A							
3	290 65	00	290 65	172 54	95 06	1 80	1 80	A							
4	419 53	00	419 53	173 81	95 13	3 75	3 75	A							
5	680 02	00	680 02	175 77	95 24	6 78	6 78	A							
6	910 34	00	910 34	178 60	95 40	11 14	11 14	A							
7	1099 54	00	1099 54	182 17	95 59	18 78	18 78	A							
8	1244 87	00	1244 87	186 30	95 82	30 48	30 48	A							
9	1344 93	00	1344 93	190 83	96 06	44 29	44 29	A							
10	1371 00	00	1371 00	195 53	96 32	61 51	61 51	A							
11	1329 87	00	1329 87	200 14	96 57	79 30	79 30	A							
12	1180 43	00	1180 43	204 34	96 80	97 50	97 50	A							
13	1021 40	00	1021 40	207 92	97 00	113 03	113 03	A							
14	850 02	00	850 02	210 86	97 16	127 47	127 47	A							
15	619 69	00	619 69	213 03	97 28	138 14	138 14	A							
16	409 93	00	409 93	214 37	97 35	144 76	144 76	A							
17	329 04	00	329 04	215 17	97 40	148 71	148 71	A							
18	200 17	00	200 17	215 59	97 42	150 74	150 74	A							
19	109 68	00	109 68	215 60	97 42	150 82	150 82	A							
20	49 36	00	49 36	215 35	97 41	149 56	149 56	A							
21	00	00	00	214 90	97 38	147 37	147 37	A							

.....

VOLUME DA HIDROGRAMA AFLUENTE (E\*06 M3) = 49 10

*49 km<sup>3</sup>*

## ANEXO 2.3

**BARRAGEM DO RIO ARACOIARA - Amortecimento de cheias**  
 Soleira livre L=20 m; Verificação para T=10 000 anos, chuva d=24 horas

**HIDROGRAMA PADRAO (Q/QFICO) AFLUENTE**

007	044	130	270	450	656	834	950	1 000	997	944	862	759	643	532
439	373	327	297	270	243	216	187	159	132	106	081	057	038	024
016	009													

**DADOS DA CURVA COZA-VOLUME (10\*\*6 M3)**

65 000	000	70 000	1 090	75 000	8 060	80 000	26 040
85 000	57 020	90 000	104 350	95 000	171 380	100 000	262 780

**DADOS DE COZA-VAZAO (M3/S)**

95 000	000	95 500	14 100	96 000	40 000	96 500	73 500
97 000	113 100	97 500	158 100	98 000	207 900	98 500	261 900
99 000	320 000						

\*\*\*\*\*  
 BARRAGEM DO RIO ARACOIARA - Amortecimento de cheias  
 Soleira livre L=20 m; Verificação para T=10 000 anos, chuva d=24 horas  
 \*\*\*\*\*

* TEMPO * (HORAS)	* VAZAO *		* AFLUENTE *		* VOLUME *		* M A *	* DESCARGA *	* GRAFICO DE VAZOS *					* (A)-AFLUENTE TOTAL *		
	* AFLUENTE * (M3/S)	* INTERMED * (M3/S)	* TOTAL * (M3/S)	* RESERVA * (M3/S)	* 0	* 500			* 1000	* 1500	* 2000	* 2500	(A)	(D)		
00	15 81	00	15 81	171 38	95 00	00	00									
1	39 40	00	39 40	171 38	95 01	32	32	A								
2	293 87	00	293 87	172 29	95 05	1 41	1 41	D	A							
3	609 93	00	609 93	175 91	95 14	3 90	3 90	D								
4	1016 55	00	1016 55	176 81	95 30	8 38	8 38	D								
5	1481 90	00	1481 90	181 27	95 54	16 22	16 22	D								
6	1884 01	00	1884 01	187 24	95 87	33 14	33 14	D								
7	2146 05	00	2146 05	194 34	96 26	57 14	57 14	D								
8	2259 00	00	2259 00	202 01	96 68	87 41	87 41	D								
9	2252 22	00	2252 22	209 75	97 10	122 04	122 04	D								
10	2132 30	00	2132 30	217 14	97 50	159 44	159 44	D								
11	1947 26	00	1947 26	223 85	97 87	194 99	194 99	D								
12	1714 58	00	1714 58	229 68	98 19	229 36	229 36	D								
13	1452 34	00	1452 34	234 31	98 45	256 88	256 88	D								
14	1201 79	00	1201 79	238 32	98 66	280 73	280 73	D								
15	991 70	00	991 70	241 23	98 82	299 19	299 19	D								
16	842 61	00	842 61	243 43	98 94	313 17	313 17	D								
17	738 69	00	738 69	245 13	99 03	323 98	323 98	D								
18	670 92	00	670 92	246 48	99 11	332 59	332 59	D								
19	609 93	00	609 93	247 58	99 17	339 36	339 36	D								
20	548 94	00	548 94	248 43	99 22	344 98	344 98	D								
21	487 94	00	487 94	249 05	99 25	348 91	348 91	D								
22	422 43	00	422 43	249 43	99 27	351 31	351 31	DA								
23	359 18	00	359 18	249 57	99 28	352 21	352 21	D								
24	298 13	00	298 13	249 48	99 27	351 68	351 68	AD								
25	239 43	00	239 43	249 19	99 26	349 80	349 80	A D								
26	182 98	00	182 98	248 69	99 23	346 66	346 66	A D								
27	128 76	00	128 76	248 02	99 19	342 85	342 85	A D								
28	85 84	00	85 84	247 18	99 15	337 03	337 03	A D								
29	54 22	00	54 22	246 23	99 09	330 99	330 99	A D								
30	36 14	00	36 14	245 21	99 04	324 52	324 52	A D								
31	20 33	00	20 33	244 16	98 98	317 82	317 82	A D								

\*\*\*\*\*  
 VOLUME DA HIDROGRAMA AFLUENTE (E\*06 M3) = 97 95



## ANEXO 2.4

**BARRAGEM DO RIO ARACOIARA - Dimensionamento do vertedouro**  
 Tulipa D=8 m; Descarga de projeto T=1 000 anos, chuva d= 8 horas

**HIDROGRAMA PADRAO (Q/QFICO) AFLUENTE**

000	036	095	212	306	496	664	802	908	981	1 000	970	861	745	620
452	299	240	146	080	036	000								

**DADOS DA CURVA COTA-VOLUME (10\*\*6 M3)**

65.000	000	70 000	1 090	75 000	8 060	80 000	26 040
85 000	57 020	90 000	104 350	95 000	171 380	100 000	262 780

**DADOS DE COTA-VAZAO (M3/S)**

95 000	000	95 800	38 700	96 200	68 400	96 600	100 200
97 000	131 500	97 400	159 800	97 800	182 500	98 200	198 500
99 000	225 200	100 000	210 000				

PAGINA 3

BARRAGEM DO RIO ARACOIARA - Dimensionamento do vertedouro  
 Tulipa D=8 m; Descarga de projeto T=1 000 anos chuva d= 8 horas

* TEMPO * * (HORAS) *	* VAZAO * * AFLUENTE * * (M3/S) *	* VAZAO * * INTERMED * * (M3/S) *	* AFLUENTE * * TOTAL * * (M3/S) *	* VOLUME * * RESERVAT * * (M06 M3) *	* N A * * RESERVAT * * (M) *	* DESCARGA * * (M3/S) *	* GRAFICO DE VAZORES * * ( M3/S) *							
							0	300	600	900	1200	1500		
* 00 *	* 00 *	* 00 *	* 00 *	* 171 38 *	* 95 00 *	* 00 *D								
* 1 00 *	* 49 36 *	* 00 *	* 49 36 *	* 171 47 *	* 95 00 *	* 23 *D A								
* 2 00 *	* 130 24 *	* 00 *	* 130 24 *	* 171 79 *	* 95 02 *	* 1 08 *D A								
* 3 00 *	* 290 65 *	* 00 *	* 290 65 *	* 172 54 *	* 95 06 *	* 3 07 *D								
* 4 00 *	* 419 53 *	* 00 *	* 419 53 *	* 173 80 *	* 95 13 *	* 6 41 *D								
* 5 00 *	* 680 02 *	* 00 *	* 680 02 *	* 175 75 *	* 95 24 *	* 11 56 *D								
* 6 00 *	* 910 34 *	* 00 *	* 910 34 *	* 178 56 *	* 95 39 *	* 18 99 *D								
* 7 00 *	* 1099 54 *	* 00 *	* 1099 54 *	* 182 09 *	* 95 59 *	* 28 34 *D								
* 8 00 *	* 1244 87 *	* 00 *	* 1244 87 *	* 186 19 *	* 95 81 *	* 39 44 *D								
* 9 00 *	* 1344 95 *	* 00 *	* 1344 95 *	* 190 67 *	* 96 06 *	* 57 67 *D								
* 10 00 *	* 1371 00 *	* 00 *	* 1371 00 *	* 195 32 *	* 96 31 *	* 77 12 *D								
* 11 00 *	* 1329 87 *	* 00 *	* 1329 87 *	* 199 87 *	* 96 56 *	* 94 90 *D								
* 12 00 *	* 1180 43 *	* 00 *	* 1180 43 *	* 204 01 *	* 96 78 *	* 114 66 *D								
* 13 00 *	* 1021 40 *	* 00 *	* 1021 40 *	* 207 53 *	* 96 98 *	* 129 74 *D								
* 14 00 *	* 850 02 *	* 00 *	* 850 02 *	* 210 41 *	* 97 14 *	* 141 05 *D								
* 15 00 *	* 619 69 *	* 00 *	* 619 69 *	* 212 53 *	* 97 25 *	* 149 27 *D								
* 16 00 *	* 408 93 *	* 00 *	* 408 93 *	* 213 84 *	* 97 32 *	* 154 33 *D								
* 17 00 *	* 329 04 *	* 00 *	* 329 04 *	* 214 61 *	* 97 36 *	* 157 31 *D								
* 18 00 *	* 200 17 *	* 00 *	* 200 17 *	* 214 99 *	* 97 39 *	* 158 79 *D A								
* 19 00 *	* 109 68 *	* 00 *	* 109 68 *	* 214 98 *	* 97 38 *	* 158 74 *AD								
* 20 00 *	* 49 36 *	* 00 *	* 49 36 *	* 214 69 *	* 97 37 *	* 157 64 *A D								
* 21 00 *	* 00 *	* 00 *	* 00 *	* 214 22 *	* 97 34 *	* 155 80 *A D								

VOLUME DA HIDROGRAMA AFLUENTE (E\*06 M3) = 49 10

## ANEXO 2.5

**BARRAGEM DO RIO ARACOIARA - Amortecimento de cheias**  
 Tulipa D=6 m. Verificação para T=10 000 anos, chuva d=24 horas

**HIDROGRAMA PADRAO (Q/QPICO) AFLUENTE**

007	044	130	270	450	656	834	950	1 000	997	944	862	759	643	532
439	373	327	297	270	243	216	187	159	132	106	081	057	038	024
016	009													

**DADOS DA CURVA COTA-VOLUME (10\*\*6 M3)**

65 000	000	70 000	1 090	75 000	8 060	80 000	26 040
85 000	57 020	90 000	104 350	95 000	171 380	100 000	262 780

**DADOS DE COTA-VAZAO (M3/S)**

95 000	000	95 800	38 700	96 200	68 400	96 600	100 200
97 000	131 500	97 400	159 800	98 200	182 500	98 600	198 500
99 000	225 200	100 000	210 000				

PAGINA 3

BARRAGEM DO RIO ARACOIARA - Amortecimento de cheias  
 Tulipa D=6 m Verificação para T=10 000 anos chuva d=24 horas

* TEMPO * (HORAS)	* VAZAO * (M3/S)	* VAZAO * (M3/S)	* AFLUENTE * (M3/S)	* VOLUME * (M3)	* N A * (M)	* DESCARGA * (M3/S)	* GRAFICO DE VAZAO * (M3/S)				* (A)=AFLUENTE TOTAL * (D)=DEFLUENTE *
							0	500	1000	1500	
* 00 *	* 15 81 *	* 00 *	* 15 81 *	* 171 38 *	* 95 00 *	* 00 *D					
* 1 00 *	* 99 40 *	* 00 *	* 99 40 *	* 171 59 *	* 95 01 *	* 55 *D A					
* 2 00 *	* 293 67 *	* 00 *	* 293 67 *	* 172 29 *	* 95 05 *	* 2 40 *D	A				
* 3 00 *	* 609 93 *	* 00 *	* 609 93 *	* 173 90 *	* 95 14 *	* 6 67 *D		A			
* 4 00 *	* 1016 55 *	* 00 *	* 1016 55 *	* 176 79 *	* 95 30 *	* 14 31 *D			A		
* 5 00 *	* 1481 90 *	* 00 *	* 1481 90 *	* 181 21 *	* 95 54 *	* 26 02 *D				A	
* 6 00 *	* 1884 01 *	* 00 *	* 1884 01 *	* 187 15 *	* 95 86 *	* 43 35 *D					A
* 7 00 *	* 2146 05 *	* 00 *	* 2146 05 *	* 194 20 *	* 96 25 *	* 72 23 *D					A
* 8 00 *	* 2259 00 *	* 00 *	* 2259 00 *	* 201 81 *	* 96 66 *	* 105 24 *D					A
* 9 00 *	* 2252 22 *	* 00 *	* 2252 22 *	* 209 49 *	* 97 08 *	* 137 49 *D					A
* 10 00 *	* 2132 50 *	* 00 *	* 2132 50 *	* 216 83 *	* 97 49 *	* 162 26 *D					A
* 11 00 *	* 1947 26 *	* 00 *	* 1947 26 *	* 223 57 *	* 97 86 *	* 172 72 *D					A
* 12 00 *	* 1714 58 *	* 00 *	* 1714 58 *	* 229 53 *	* 98 18 *	* 181 96 *D					A
* 13 00 *	* 1452 54 *	* 00 *	* 1452 54 *	* 234 86 *	* 98 46 *	* 192 75 *D					A
* 14 00 *	* 1201 79 *	* 00 *	* 1201 79 *	* 239 83 *	* 98 68 *	* 203 76 *D					A
* 15 00 *	* 991 70 *	* 00 *	* 991 70 *	* 241 82 *	* 98 85 *	* 213 42 *D					A
* 16 00 *	* 842 61 *	* 00 *	* 842 61 *	* 244 33 *	* 98 99 *	* 224 58 *D					A
* 17 00 *	* 738 49 *	* 00 *	* 738 49 *	* 246 37 *	* 99 10 *	* 223 64 *D					A
* 18 00 *	* 670 92 *	* 00 *	* 670 92 *	* 248 11 *	* 99 20 *	* 222 20 *D					A
* 19 00 *	* 609 93 *	* 00 *	* 609 93 *	* 249 61 *	* 99 28 *	* 220 95 *D					A
* 20 00 *	* 548 94 *	* 00 *	* 548 94 *	* 250 91 *	* 99 35 *	* 219 87 *D					A
* 21 00 *	* 487 94 *	* 00 *	* 487 94 *	* 251 98 *	* 99 41 *	* 218 98 *D					A
* 22 00 *	* 422 43 *	* 00 *	* 422 43 *	* 252 83 *	* 99 46 *	* 218 27 *D					A
* 23 00 *	* 359 18 *	* 00 *	* 359 18 *	* 253 46 *	* 99 49 *	* 217 75 *D					A
* 24 00 *	* 298 19 *	* 00 *	* 298 19 *	* 253 86 *	* 99 51 *	* 217 42 *D					A
* 25 00 *	* 239 45 *	* 00 *	* 239 45 *	* 254 04 *	* 99 52 *	* 217 27 *DA					A
* 26 00 *	* 182 98 *	* 00 *	* 182 98 *	* 254 02 *	* 99 52 *	* 217 29 *D					A
* 27 00 *	* 128 76 *	* 00 *	* 128 76 *	* 253 80 *	* 99 51 *	* 217 47 *AD					A
* 28 00 *	* 85 84 *	* 00 *	* 85 84 *	* 253 40 *	* 99 49 *	* 217 80 *A D					A
* 29 00 *	* 54 22 *	* 00 *	* 54 22 *	* 252 87 *	* 99 46 *	* 218 24 *A D					A
* 30 00 *	* 36 14 *	* 00 *	* 36 14 *	* 252 24 *	* 99 42 *	* 218 76 *A D					A
* 31 00 *	* 20 33 *	* 00 *	* 20 33 *	* 251 55 *	* 99 39 *	* 219 33 *A D					A

VOLUME DA HIDROGRAMA AFLUENTE (R\*06 M3) = 97 95

## ANEXO 2.6

**BARRAGEM DO RIO ARACOIARA - DESVIO 1a CHEIA**  
**RETECA h=90 m. Descarga de projeto T=50 anos, chuva d= 8 horas**

**HIDROGRAMA PADRAO (Q/QPICO) AFLUENTE**

000	036	095	212	306	496	664	802	908	961	1 000
970	861	745	620	452	299	240	146	080	036	000

**DADOS DA CURVA COTA-VOLUME (10\*\*6 M3)**

65 000	000	70 000	1 090	75 000	8 060	80 000	26 040
85 000	57 020	90 000	104 350	95 000	171 380	100 000	262 780

**DADOS DE COTA-VAZAO (M3/S)**

70 0 000	71 750	208 400	73 500	589 300	75 250	1082 600
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**BARRAGEM DO RIO ARACOIARA - DESVIO 1a CHEIA**  
**RETECA h=90 m. Descarga de projeto T=50 anos, chuva d= 8 horas**  
 .....

* TEMPO * * (HORAS) *	* VAZAO *		* AFLUENTE TOTAL * (M3/S)	* VOLUME * (1006 M3)		* M A * (M)	* DESCARGA * (M3/S)	* GRAFICO DE VAZAO * (M3/S)				* (A)=AFLUENTE TOTAL * * (D)=DEFLUENTE *	
	* AFLUENTE * (M3/S)	* INTERMED * (M3/S)		* RESERVA * (M3/S)	* RESERVA * (M3/S)			0	200	400	600	800	1000
* 00 *	* 00 *	* 00 *	* 00 *	* 1 09 *	* 70 00 *	* 70 00 *	* 00 *					* D *	
* 1 00 *	* 29 81 *	* 00 *	* 29 81 *	* 1 14 *	* 70 03 *	* 70 03 *	* 3 97 *	* DA				* *	
* 2 00 *	* 78 66 *	* 00 *	* 78 66 *	* 1 29 *	* 70 15 *	* 70 15 *	* 17 37 *	* D A				* *	
* 3 00 *	* 175 54 *	* 00 *	* 175 54 *	* 1 64 *	* 70 39 *	* 70 39 *	* 46 62 *	* D A				* *	
* 4 00 *	* 253 37 *	* 00 *	* 253 37 *	* 2 16 *	* 70 77 *	* 70 77 *	* 91 35 *	* D A				* *	
* 5 00 *	* 410 69 *	* 00 *	* 410 69 *	* 2 91 *	* 71 31 *	* 71 31 *	* 155 51 *	* D A				* *	
* 6 00 *	* 549 79 *	* 00 *	* 549 79 *	* 3 88 *	* 72 00 *	* 72 00 *	* 253 82 *	* D A				* *	
* 7 00 *	* 664 06 *	* 00 *	* 664 06 *	* 4 85 *	* 72 70 *	* 72 70 *	* 414 36 *	* D A				* *	
* 8 00 *	* 751 82 *	* 00 *	* 751 82 *	* 5 67 *	* 73 29 *	* 73 29 *	* 543 18 *	* D A				* *	
* 9 00 *	* 812 27 *	* 00 *	* 812 27 *	* 6 32 *	* 73 75 *	* 73 75 *	* 660 69 *	* D A				* *	
* 10 00 *	* 829 00 *	* 00 *	* 829 00 *	* 6 74 *	* 74 06 *	* 74 06 *	* 745 79 *	* D A				* *	
* 11 00 *	* 803 16 *	* 00 *	* 803 16 *	* 6 93 *	* 74 19 *	* 74 19 *	* 783 04 *	* D A				* *	
* 12 00 *	* 712 91 *	* 00 *	* 712 91 *	* 6 86 *	* 74 14 *	* 74 14 *	* 769 69 *	* D A				* *	
* 13 00 *	* 616 86 *	* 00 *	* 616 86 *	* 6 58 *	* 73 94 *	* 73 94 *	* 713 75 *	* D A				* *	
* 14 00 *	* 513 36 *	* 00 *	* 513 36 *	* 6 19 *	* 73 66 *	* 73 66 *	* 634 42 *	* D A				* *	
* 15 00 *	* 374 26 *	* 00 *	* 374 26 *	* 5 67 *	* 73 29 *	* 73 29 *	* 542 76 *	* D A				* *	
* 16 00 *	* 247 57 *	* 00 *	* 247 57 *	* 5 02 *	* 72 82 *	* 72 82 *	* 441 03 *	* D A				* *	
* 17 00 *	* 198 72 *	* 00 *	* 198 72 *	* 4 41 *	* 72 38 *	* 72 38 *	* 345 43 *	* D A				* *	
* 18 00 *	* 120 89 *	* 00 *	* 120 89 *	* 3 89 *	* 72 01 *	* 72 01 *	* 263 98 *	* D A				* *	
* 19 00 *	* 66 24 *	* 00 *	* 66 24 *	* 3 39 *	* 71 65 *	* 71 65 *	* 196 74 *	* D A				* *	
* 20 00 *	* 29 81 *	* 00 *	* 29 81 *	* 2 93 *	* 71 32 *	* 71 32 *	* 157 10 *	* D A				* *	
* 21 00 *	* 00 *	* 00 *	* 00 *	* 2 49 *	* 71 00 *	* 71 00 *	* 119 20 *	* D A				* *	

.....

**VOLUME DA HIDROGRAMA AFLUENTE (10\*\*06 M3) = 29 66**

## ANEXO 2.7

**BARRAGEM DO RIO ARACOIAMA - Dimensionamento ESTRUTURA DE DESVIO**  
 1 galerias (3,5m x 3,5m) Descarga de projeto T=50 anos, chuva d= 8 horas

**HIDROGRAMA PADRAO (Q/QPICO) AFLUENTE**

000	036	095	212	306	496	664	802	908	981	1 000
970	861	745	620	452	299	240	146	080	036	000

**DADOS DA CURVA COTA-VOLUME (10\*\*6 M3)**

65 000	000	70 000	1 090	75 000	8 060	80 000	26 040
85 000	57 020	90 000	104 350	95 000	171 380	100 000	262 780

**DADOS DE COTA-VAZAO (M3/S)**

70 000	000	71 750	6 000	73 500	17 200	75 250	32 700
77 500	40 500	78 750	47 300	80 500	53 500		

.....

**BARRAGEM DO RIO ARACOIAMA - Dimensionamento ESTRUTURA DE DESVIO**  
 1 galerias (3,5m x 3,5m) Descarga de projeto T=50 anos chuva d= 8 horas

.....

* TEMPO * * (HORAS) *	* VAZAO *		* AFLUENTE * * (M3/S) *	* VOLUME * * (10 <sup>6</sup> M3) *	* N A * * (M) *	* DESCARGA * * (M3/S) *	* GRAFICO DE VAZAO * * (M3/S) *							
	* AFLUENTE * * (M3/S) *	* INTERMED * * (M3/S) *					* TOTAL * * (M3/S) *	* RESERVAT * * (M3/S) *	0	200	400	600		
* 00 *	00	00	00	00	1 09	70 00	00	D						
* 1 00 *	29 81	00	29 81	1 14	70 04	13	Da							
* 2 00 *	78 66	00	78 66	1 34	70 18	61	D	A						
* 3 00 *	175 54	00	175 54	1 79	70 50	1 72	D		A					
* 4 00 *	253 37	00	253 37	2 55	71 05	3 60	D			A				
* 5 00 *	410 69	00	410 69	3 73	71 89	6 92	D				A			
* 6 00 *	549 79	00	549 79	5 42	73 11	14 68	D					A		
* 7 00 *	664 06	00	664 06	7 53	74 62	27 12	D						A	
* 8 00 *	751 82	00	751 82	9 97	75 53	33 67	D							A
* 9 00 *	812 27	00	812 27	12 66	76 28	36 27	D							A
* 10 00 *	828 00	00	828 00	15 48	77 06	38 98	D							A
* 11 00 *	803 16	00	803 16	18 27	77 84	42 34	D							A
* 12 00 *	712 91	00	712 91	20 84	78 55	46 23	D							A
* 13 00 *	616 86	00	616 86	23 06	79 17	48 79	D							A
* 14 00 *	513 36	00	513 36	24 91	79 69	50 62	D							A
* 15 00 *	374 26	00	374 26	26 33	80 05	51 89	D							A
* 16 00 *	247 57	00	247 57	27 26	80 20	52 42	D							A
* 17 00 *	198 72	00	198 72	27 87	80 30	52 78	D							A
* 18 00 *	120 89	00	120 89	28 26	80 36	53 00	D	A						
* 19 00 *	66 24	00	66 24	28 40	80 38	53 08	D							
* 20 00 *	29 81	00	29 81	28 38	80 38	53 07	A	D						
* 21 00 *	00	00	00	28 25	80 36	52 99	A	D						

.....

**VOLUME DA HIDROGRAMA AFLUENTE (10<sup>6</sup> M3) = 29 66**

### **3 - QUANTITATIVOS DAS OBRAS DE TERRA**

Os quantitativos para as obras de terra tiveram como base os seguintes desenhos de projeto

- AR-GR-1001 - Arranjo Geral - Alternativa 1 - Planta
- AR-GR-1005 - Esquema de Desvio - Alternativas 1, 2, 3 e 4
- AR-BT-3005 - Seções Típicas e Perfil Longitudinal de Saneamento - Alternativas 1, 2, 3 e 4
- AR-BT-3006 - Seções Típicas da Barragem - Alternativas 1, 2, 3 e 4
- AR-BT-3007 - Anteprojeto - Seções Transversais da Barragem
- AR-GR-1006 - Anteprojeto - Arranjo Geral - Planta
- AR-GR-1007 - Anteprojeto - Esquema de Desvio e Cronograma Físico

Foram calculados os volumes dos materiais de acordo com a memória de cálculo apresentada a seguir

# BARRAGEM ARACOIABA

## Cálculo de Volumes

### 1. BARRAGEM DA MARGEM DIREITA

1.1. Seção típica entre as estacas 0+0,00 a 25+0,00

a) cota fundação = 98,50

$$\textcircled{S} (4+10)/2 \times 1,5 = 10,5 \text{ m}^2$$

$$\textcircled{T} 1 \times 1,5 = 1,5 \text{ m}^2$$

$$\textcircled{RR} 2 \times 1,5 = 3,0 \text{ m}^2$$

b) cota 97,50

$$\textcircled{S} 4+14/2 \times 2,5 = 22,5 \text{ m}^2$$

$$\textcircled{T} 1 \times 2,5 = 2,5 \text{ m}^2$$

$$\textcircled{RR} 2 \times 2,5 = 5,0 \text{ m}^2$$

1.2. Seção típica entre 25+0,00 e 33+0,00

a) cota 97,50 (escavação) e 91,00 (cut-off)

$$\textcircled{S} [(4+26,5)/2 \times 5,5] + [(19+14)/2 \times 5,5] - [10 \times 5] = 126,7 \text{ m}^2$$

$$\textcircled{T} 1 \times 6,5 = 6,5 \text{ m}^2$$

$$\textcircled{RR} 2 \times 6,5 = 13,0 \text{ m}^2$$

$$\textcircled{F} (1 \times 5) + (1,5 \times 9) = 18,5 \text{ m}^2$$

b) cota 81,00 (eixo)

$$\textcircled{S} [(4+27)/2 \times 5] + [(27+45)/2 \times 5] + [48+25/2 \times 5] + (25 \times 5,5/2) = 833,0 \text{ m}^2$$

$$\textcircled{F} (1 \times 15) + (1,5 \times 44) = 81,0 \text{ m}^2$$

$$\textcircled{T} 1 \times 16 = 16 \text{ m}^2$$

$$\textcircled{RR} 2 \times 10 = 20 \text{ m}^2$$

$$\textcircled{E} 2 \times 6 = 12 \text{ m}^2$$

c) cota 69,00 - Est 39+0,00

$$\textcircled{S} \left[ \frac{(4+24)}{2} \times 5 \right] + \left[ \frac{(24+46)}{2} \times 5 \right] + \left[ \frac{(49+95)}{2} \times 10 \right] + \left[ \frac{(98+147)}{2} \times 11 \right] - 126 = 2106,5 \text{ m}^2$$

$$\textcircled{F} (1 \times 27) + (1,5 \times 66) = 126 \text{ m}^2$$

$$\textcircled{T} 1 \times 31 = 31 \text{ m}^2$$

$$\textcircled{RR} 2 \times 10 = 20 \text{ m}^2$$

$$\textcircled{E} 2 \times 21 = 42 \text{ m}^2$$

10 UNO BARRIGAS (BARRIGAS) CIRCUNDA

$$\textcircled{S} \left( \frac{10,5}{2} \times 20 \right) + \left[ \frac{(0,5+22,5)}{2} \times 70 \right] + (22,5 \times 300) + (26,7 \times 50) + \left[ \frac{(26,7+833)}{2} \times 80 \right] + \left[ \frac{(833+2186,5)}{2} \times 145 \right] = 27212 \text{ m}^3$$

$$\textcircled{F} (185 \times 50) + \left[ \frac{(18,5+81)}{2} \times 80 \right] + \left[ \frac{(81+126)}{2} \times 45 \right] = 12915 \text{ m}^3$$

$$\textcircled{T} \left( \frac{1,5}{2} \times 20 \right) + \left[ \frac{(1,5+2,5)}{2} \times 70 \right] + [2,5 \times 300] + [6,5 \times 50] + \left[ \frac{(6,5+16)}{2} \times 80 \right] + \left[ \frac{(16+8)}{2} \times 145 \right] = 5613 \text{ m}^3$$

$$\textcircled{RR} \left( \frac{3}{2} \times 120 \right) + \left[ \frac{(3+5)}{2} \times 80 \right] + [5 \times 300] + [13 \times 50] + \left[ \frac{(3+20)}{2} \times 80 \right] + [20 \times 145] = 6870 \text{ m}^3$$

$$\textcircled{E} \left[ \frac{12}{2} \times 80 \right] + \left[ \frac{(12+42)}{2} \times 145 \right] = 4535 \text{ m}^3$$

TOTAL = 308963 m<sup>3</sup>

Dreno de Pe' (l ≈ 270)

$$\textcircled{T} [(0,5 \times 5) \times 270] = 675 \text{ m}^3$$

$$\textcircled{E} \left[ \left( \frac{5 \times 2}{2} + 1 \times 1 \right) \times 270 \right] = 1620 \text{ m}^3$$

## 2. BARRAGEM DA MARGEM ESQUERDA

2.1. Seção típica entre as estacas 90+0,00 a 100,00

a) cota 96,60

$$\textcircled{S} [(4+20)/2 \times 4] = 48 \text{ m}^2$$

$$\textcircled{T} 1 \times 4 = 4 \text{ m}^2$$

$$\textcircled{RR} 2 \times 4 = 8 \text{ m}^2$$

b) cota 93,50

$$\textcircled{S} [(4+31)/2 \times 6,5] - 21 = 93 \text{ m}^2$$

$$\textcircled{F} (1,5 \times 13) + (1 \times 1,5) = 21 \text{ m}^2$$

$$\textcircled{T} 1 \times 6 = 6 \text{ m}^2$$

$$\textcircled{RR} 2 \times 6 = 12 \text{ m}^2$$

2.2 Seção típica entre estacas 75+000 a 90+000

a) cota 21,50 - est 85+000

$$\textcircled{S} [(4+39)/2 \times 8,5] - 16 + 2 = 169 \text{ m}^2$$

$$\textcircled{F} (1 \times 4) + (1,5 \times 8) = 16 \text{ m}^2$$

$$\textcircled{T} 1 \times 8,0 = 8 \text{ m}^2$$

$$\textcircled{RR} 2 \times 8 = 16$$

b) cota 30,00

$$\textcircled{S} 169 + (2,5 \times 44) + 4 = 283 \text{ m}^2$$

$$\textcircled{F} 16 + (1,5 \times 4) + (1 \times 2,5) = 24,5 \text{ m}^2$$

$$\textcircled{T} 8 + (1 \times 2) = 10 \text{ m}^2$$

$$\textcircled{RR} 16 + (2 \times 2) = 20 \text{ m}^2$$



c) cota 92,00

$$\textcircled{S} \left[ \frac{(4+38)}{2} \times 8 \right] + 5 - 16 = 157 \text{ m}^2$$

$$\textcircled{F} (1 \times 4) + (1,5 \times 13) = 23,5 \text{ m}^2$$

$$\textcircled{T} 1 \times 8 = 8 \text{ m}^2$$

$$\textcircled{RR} 2 \times 8 = 16 \text{ m}^2$$

2.3 Seção típica entre as estacas 63+000 a 73+0,00

a) cota 93,00 - est. 73+0,00

$$\textcircled{S} \frac{(4+32)}{2} \times 7 = 126 - 19,5 = 106,5 \text{ m}^2$$

$$\textcircled{T} 1 \times 6,5 = 6,5 \text{ m}^2 \quad \textcircled{F} (1 \times 3 + 4 + 5 \times 1) = 19,5 \text{ m}^2$$

$$\textcircled{RR} 2 \times 6,5 = 13 \text{ m}^2$$

b) cota 96,00 - est 69+000

$$\textcircled{S} \left[ \frac{(4+21)}{2} \times 4 \right] + 2 = 52 \text{ m}^2$$

$$\textcircled{T} (1 \times 3) = 6$$

$$\textcircled{RR} (2 \times 3) = 6$$

c) cota 92 (exa) - est 65+0,00

$$\textcircled{S} 126 + \left( \frac{35 \times 3}{2} \right) = 178,5 - 23 = 155,5 \text{ m}^2$$

$$\textcircled{T} 1 \times 6,5 = 6,5 \text{ m}^2 \quad \textcircled{F} (1 \times 5) + (1,5 \times 12) = 23 \text{ m}^2$$

$$\textcircled{RR} 2 \times 6,5 = 13,0 \text{ m}^2$$

d) cota 87 (cixa) - est 63+0,00

$$\textcircled{S} \left[ \frac{(4+52)}{2} \times 12 \right] + \left( \frac{52 \times 2,5}{2} \right) - 48,5 = 352,5 \text{ m}^2$$

$$\textcircled{F} (1 \times 9,5) + (1,5 \times 26) = 48,5 \text{ m}^2$$

$$\textcircled{T} 1 \times 12 = 12 \text{ m}^2$$

$$\textcircled{RR} 2 \times 10 = 20$$

$$\textcircled{E} 2 \times 2 = 4 \text{ m}^2$$

24. Seção típica entre estações 53+000 e 63+000

a) cota 62,50 - est. 63+0,00

$$\textcircled{S} \left[ \frac{4+24}{2} \times 5 \right] + \left[ \frac{24+46}{2} \times 5 \right] + \left[ \frac{49+69}{2} \times 4,5 \right] + \left[ \frac{19+14}{2} \times 3 \right] + 5 - 56 = 509 \text{ m}^2$$

$$\textcircled{F} (1 \times 12,5) + (1,5 \times 29) = 56 \text{ m}^2$$

$$\textcircled{T} 1 \times 13 = 13 \text{ m}^2$$

$$\textcircled{RR} 2 \times 10 = 20 \text{ m}^2$$

$$\textcircled{E} 2 \times 3 = 6 \text{ m}^2$$

b) cota 76,00 - est. 58+0,00

$$\textcircled{S} \left[ \frac{4+20}{2} \times 5 \right] + \left[ \frac{24+46}{2} \times 5 \right] + \left[ \frac{49+69}{2} \times 4,5 \right] + \left[ \frac{19+14}{2} \times 3 \right] + \left[ \frac{35}{2} \times 2,5 \right] + \left[ \frac{8+14}{2} \times 2,5 \right] - 600 = 212,50 \text{ m}^2$$

$$\textcircled{F} (1 \times 20) + (1,5 \times 30) = 85 \text{ m}^2$$

$$\textcircled{T} 1 \times 22 = 22 \text{ m}^2$$

$$\textcircled{RR} 2 \times 10 = 20 \text{ m}^2$$

$$\textcircled{E} 2 \times 2 = 4 \text{ m}^2$$

c) cota 68,50 - est. 53+0,00

$$\textcircled{S} \left[ \frac{4+24}{2} \times 5 \right] + \left[ \frac{20+46}{2} \times 5 \right] + \left[ \frac{49+69}{2} \times 4,5 \right] + \left[ \frac{17+14}{2} \times 1 \right] + (7 \times 2) - 125,5 = 2207 \text{ m}^2$$

$$\textcircled{F} (28 \times 1) + (1,5 \times 65) = 125,5 \text{ m}^2$$

$$\textcircled{T} 1 \times 22 = 22 \text{ m}^2$$

$$\textcircled{RR} 2 \times 10 = 20 \text{ m}^2$$

$$\textcircled{E} 2 \times 19 = 38 \text{ m}^2$$

10. JMC BARRASOM (FRAGEN CSAUERDA

$$\textcircled{S} \left[ \frac{48}{2} \times 90 \right] + \left[ \frac{(48+93)}{2} \times 100 \right] + \left[ \frac{(93+169)}{2} \times 100 \right] + \\ + \left[ \frac{(169+283)}{2} \times 100 \right] + \left[ \frac{(283+57)}{2} \times 100 \right] + \left[ \frac{(57+106,5)}{2} \times 40 \right] + \\ + \left[ \frac{(106,5+52)}{2} \times 80 \right] + \left[ \frac{(52+155,5)}{2} \times 80 \right] + \left[ \frac{(155,5+352,5)}{2} \times 40 \right] + \\ + \left[ \frac{(509+212,5)}{2} \times 100 \right] + \left[ \frac{(212,5+2207)}{2} \times 100 \right] = 554030 \text{ m}^3$$

$$\textcircled{F} \left[ \frac{21}{2} \times 100 \right] + \left[ \frac{(1+16)}{2} \times 100 \right] + \left[ \frac{(16+24,5)}{2} \times 100 \right] + \\ + \left[ \frac{(24,5+235)}{2} \times 100 \right] + \left[ \frac{(235+12,5)}{2} \times 40 \right] + \left[ \frac{(12,5+7)}{2} \times 80 \right] + \\ + \left[ \frac{(7+23)}{2} \times 80 \right] + \left[ \frac{(23+48,5)}{2} \times 40 \right] + \left[ \frac{(56+80,5)}{2} \times 100 \right] + \\ + \left[ \frac{(80,5+125,5)}{2} \times 100 \right] = 23440 \text{ m}^3$$

$$\textcircled{T} \left[ \frac{4}{2} \times 90 \right] + \left[ \frac{(4+6)}{2} \times 100 \right] + \left[ \frac{(6+8)}{2} \times 100 \right] + \left[ \frac{(8+10)}{2} \times 100 \right] + \\ + \left[ \frac{(10+5)}{2} \times 100 \right] + \left[ \frac{(5+8)}{2} \times 40 \right] + \left[ \frac{(8+6,5)}{2} \times 80 \right] + \left[ \frac{(6,5+6)}{2} \times 80 \right] + \\ + \left[ \frac{(6+6,5)}{2} \times 80 \right] + \left[ \frac{(6,5+2)}{2} \times 40 \right] + \left[ \frac{(2+22)}{2} \times 100 \right] + \left[ \frac{(22+22)}{2} \times 100 \right] = 8730 \text{ m}^3$$

$$\textcircled{RF} \left[ \frac{8}{2} \times 90 \right] + \left[ \frac{(8+12)}{2} \times 100 \right] + \left[ \frac{(12+16)}{2} \times 100 \right] + \left[ \frac{(16+20)}{2} \times 100 \right] + \\ + \left[ \frac{(20+16)}{2} \times 100 \right] + \left[ \frac{(16+13)}{2} \times 40 \right] + \left[ \frac{(13+6)}{2} \times 20 \right] + \left[ \frac{(6+13)}{2} \times 80 \right] + \\ + \left[ \frac{(13+20)}{2} \times 40 \right] + \left[ 20 \times 200 \right] = 13120 \text{ m}^3$$

Dreno de De' ( $l_1 = 660$ ;  $l_2 = 200$ )

$$\textcircled{T} \left[ (0,5 \times 2,0) \times 660 \right] + \left[ (0,5 \times 5) \times 200 \right] = 1160 \text{ m}^3$$

$$\textcircled{E} \left[ (1 \times 1,5) \times 660 \right] + \left[ \left( \frac{5 \times 2}{2} + 1 \times 1 \right) \times 200 \right] = 2190 \text{ m}^3$$

$$\textcircled{E} \left[ \frac{4}{2} \times 40 \right] + \left[ \frac{(6+24)}{2} \times 100 \right] + \left[ \frac{(24+38)}{2} \times 100 \right] = 4680 \text{ m}^3$$

### 3. BARRAGEM do Leito do Rio

3.1. Seção típica Est 37+0,00 a 53+0,00

a) cotia 69 - est 39+0,00

Ⓢ  $2\ 186,5\ m^2$

ⓕ  $1260\ m^2$

Ⓣ  $30\ m^2$

ⓂⓂ  $20,0\ m^2$

ⓔ  $42,0\ m^2$

b) cotia 68 - est. 40+0,00

Ⓢ  $[(4 + 24)/2 \times 5] + [(24 + 47)/2 \times 5] + [(49 + 94)/2 \times 10] + [(97 + 153)/2 \times 12]$   
 $+ [(34 + 24)/2 \times 5] - 1375 = 2532,5\ m^2$

ⓕ  $(1 \times 34) + (15 \times 69) = 1375\ m^2$

Ⓣ  $1 \times 25 = 25\ m^2$

ⓂⓂ  $2 \times 10 = 200\ m^2$

ⓔ  $2 \times 15 = 30\ m^2$

c) cotia 68,5 - est 53+0,00

Ⓢ  $2\ 207\ m^2$

ⓕ  $25,5\ m^2$

Ⓣ  $22,0\ m^2$

ⓂⓂ  $20,0\ m^2$

ⓔ  $38,0\ m^2$

Doers de Pé' ( $l = 300\ m$ )

Ⓣ  $(1 \times 12) \times 300 = 3\ 600\ m^3$

ⓔ  $(14 \times 5/2) \times 300 = 10\ 500\ m^3$

10 uma da SARRAGEN de 1000 m<sup>2</sup>

$$\textcircled{S} \left[ \frac{(2186,5 + 2532,5)}{2} \times 30 \right] + [2532,5 \times 130] + \left[ \frac{(2532,5 + 2207)}{2} \times 120 \right]$$
$$= 684\ 580 \text{ m}^3$$

$$\textcircled{F} \left[ \frac{(126 + 137,5)}{2} \times 30 \right] + [137,5 \times 130] + \left[ \frac{(137,5 + 125,5)}{2} \times 120 \right]$$
$$= 37\ 607 \text{ m}^3$$

$$\textcircled{T} \left[ \frac{(31 + 25)}{2} \times 30 \right] + [25 \times 130] + \left[ \frac{(25 + 22)}{2} \times 120 \right] = 6\ 910 \text{ m}^3$$

$$\textcircled{RR} \left[ \frac{(20 + 20)}{2} \times 30 \right] + [20 \times 130] + \left[ \frac{(20 + 20)}{2} \times 120 \right] = 5\ 600 \text{ m}^3$$

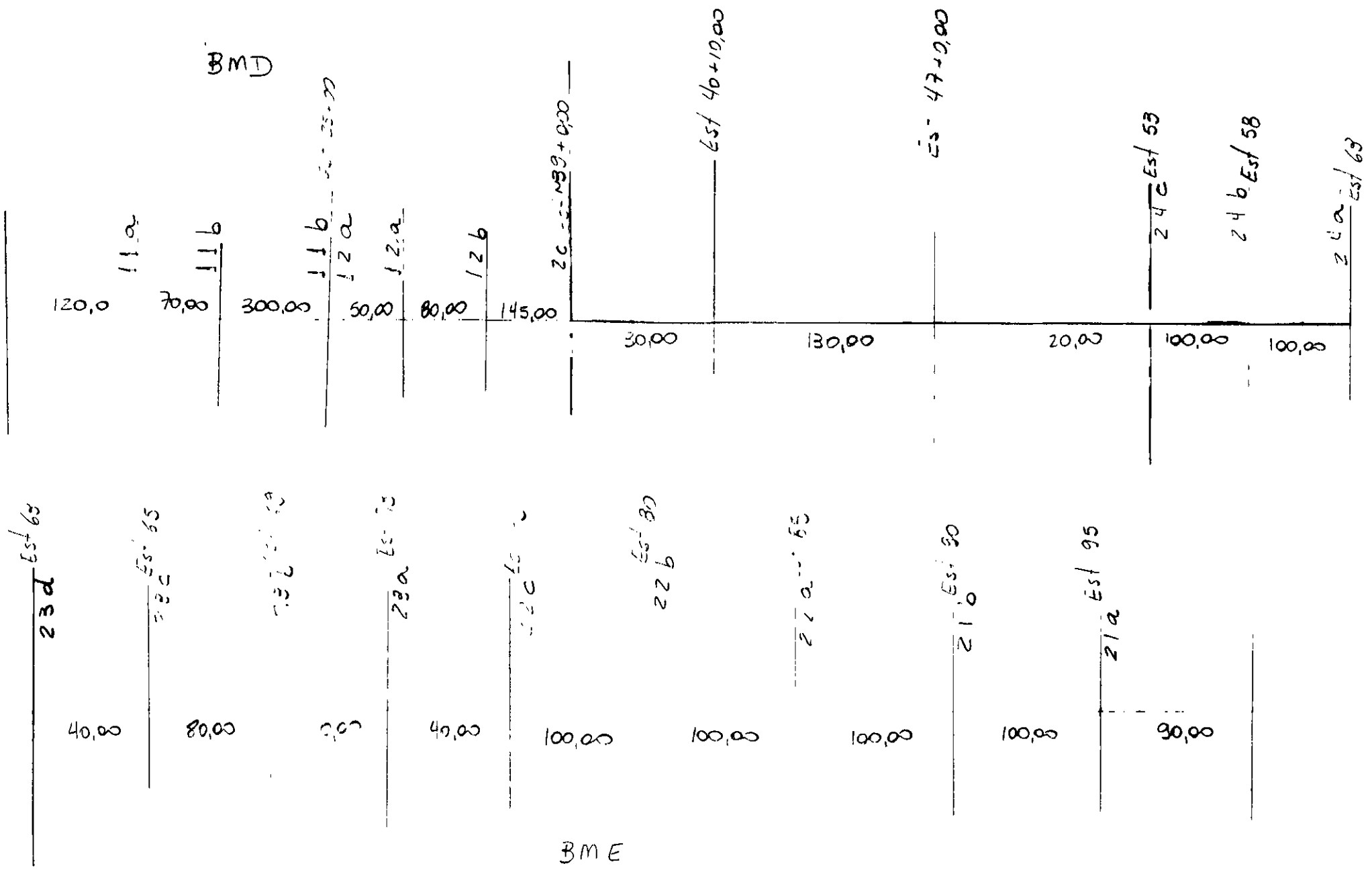
$$\textcircled{E} \left[ \frac{(42 + 30)}{2} \times 30 \right] + [30 \times 130] + \left[ \frac{(30 + 38)}{2} \times 120 \right] = 3\ 060 \text{ m}^3$$

ARACOIABA

RESUMO DE QUANTITATIVOS

Volumes Compactados  
 para a Base de Terra

MATERIAIS	BMD	BLR	BME	TOTAL	
Ⓢ	272 200	681.400	354 100	1 310.700	
ⓕ	20 000	37 600	28 500	86 100	
Ⓣ	5 600	7 000	6 800	21 400	
Ⓜ	6 900	5 600	13 200	25 700	
ⓔ	4 400	9 100	4 700	18 200	
TOTAL 1	309.100	743.700	409 300	1.462.100	
DRENHO DE PÉ	Ⓣ	700	3 600	1 200	5 500
	ⓔ	1 700	10 500	3 200	15 400
	TOTAL 2	2 400	14 100	3 400	19.900
TOTAL	311 500	771.900	412 700	1.501.900	



# VOLUMES DE ESCAVAÇÃO

## 1. Barragem de Terra

### 1.1. Desmatamento e Limpeza

$$A = (25 + 20/2 \times 450) + (30 + 25/2 \times 70) + (30 + 180/2 \times 400) + \\ (190 + 60/2 \times 330) + (60 + 30/2 \times 80) + (30 + 60/2 \times 220) + \\ (60 + 20/2 \times 400) = 124\,800 \text{ m}^2$$

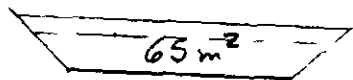
### 1.2. Escavação

#### 1.2.1. Comum

$$1500 = \left[ (25 + 20/2 \times 480) \times 0,5 \right] + \left[ (50 + 30/2 \times 55) \times 3,5 \right] + \left[ 130 \times (130 + 50/2) \times 3 \right] \\ + \left[ 160 \times 30 \times 6 \right] + \left[ 160 \times 20 \times 1 \right] + \left[ (160 + 80/2 \times 200) \times 1,70 \right] + \left[ 50 + 50/2 \times 240 \times 1 \right] \\ + \left[ 40 + 60/2 \times 300 \times 1 \right] + \left[ 40 + 20/2 \times 200 \times 0,5 \right] = 158\,200 \text{ m}^3$$

$$\text{cut-off}_{MD} = \left[ (22 + 15/2 \times 3,5) \times 60 \right] + \left[ 65/2 \times 70 \right] = 15\,873 \text{ m}^3$$

$$\text{cut-off}_{ME} = (15 + 19,5/2 \times 2,5) \times 210 = 9\,056 \text{ m}^3$$



$$V_{\text{solto}} = 183.200 \text{ m}^3$$

#### 1.2.2 - Rocha

$$V_{\text{ROCHA}} = (50 \times 50) \times 2 \times 2 = 10\,000 \text{ m}^3$$



### 1.3 - Tratamentos de Fundação

1.3.1 - Em Rocha

$$A_R = (350 \times 80) + (120 \times 90) = 38\,800 \text{ m}^2 \Rightarrow 40\,000 \text{ m}^2$$

1.3.2 - Em Solo

$$A_S = 125\,000 - 40\,000 = 85\,000 \text{ m}^2$$

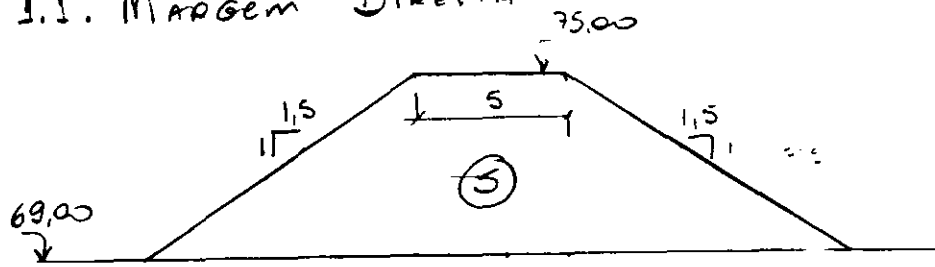
~~Proteção com GRAMA~~

$$\begin{aligned} A_{\text{GRAMA}} &= \left[ \frac{7}{2} \times 500 \right] + \left[ 7 \times 60 \right] + \left[ \frac{7+42}{2} \times 240 \right] + \left[ 42 \times 300 \right] \\ &+ \left[ \frac{42+21}{2} \times 240 \right] + \left[ \frac{21}{2} \times 160 \right] + \left[ 21 \times 300 \right] + \left[ \frac{21}{2} \times 190 \right] \\ &= 38\,185 \text{ m}^2 \end{aligned}$$

# VOLUMES DAS ENSECADDEIRAS

## 1. Ensecadeiras de 1ª FASE

### 1.1. Margem Direita



a) cota 69,00

$$\textcircled{5} \left[ \frac{(5+23)}{2} \times 6 \right] = 84 \text{ m}^2$$

$$V_{MD} = \left( \frac{84}{2} \times 180 \right) + (84 \times 410) = 42.000 \text{ m}^3$$

### 1.2. Margem Esquerda

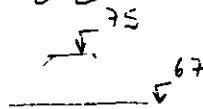
a) cota 69,00

$$\textcircled{5} 84 \text{ m}^2$$

$$V_{ME} = \left[ \frac{84}{2} \times (25+1'5) \right] + [84 \times 310] = 34.860 \text{ m}^3$$

## 2. Ensecadeiras de 2ª FASE

### 2.1. Jusante

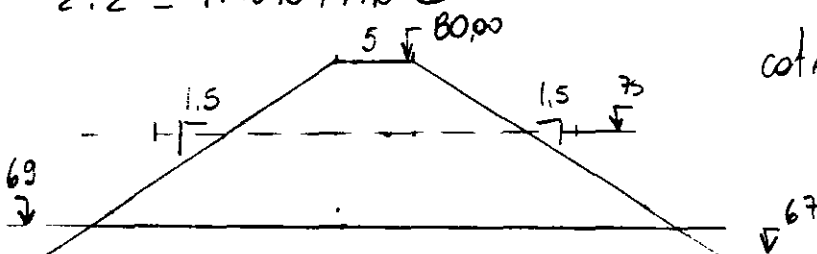


a) cota 69 - 84 m<sup>2</sup>

b) cota 67 -  $\frac{(5+20)}{2} \times 6 = 136 \text{ m}^2$

$$V_{jus} = [84 \times 60] + [136 \times 35] = 9.800 \text{ m}^3$$

### 2.2. Montante



$$\text{cota } 75 - \left[ \frac{5+20}{2} \times 5 \right] = 62,5 \text{ m}^2$$

000027

a) cota 69,00

$$\textcircled{5} \left[ 5 + 38/2 \times 11 \right] = 236,5 \text{ m}^2$$

b) cota 67,00

$$\textcircled{5} \left[ 5 + 44/2 \times 13 \right] = 318,5 \text{ m}^2$$

$$V_{\text{MONT}} = \left[ 236,5 \times (70 + 80) \right] + \left[ 318,5 \times 40 \right] + \left[ 236,5/2 \times 35 \right] \\ + \left[ 62,5 \times 40 \right] + \left[ 62,5/2 \times 15 \right] = 55 322 \text{ m}^3$$

TOTAL

$$\textcircled{5} 42 000 + 34 860 + 2 800 + 55 322 = 141 982 \text{ m}^3$$

$$\textcircled{5} 142 000$$

### 3. Remoção

$$\text{MD} - (84 \times 220) = 18 480 \text{ m}^3$$

$$\text{ME} - (84 \times 320) + \left[ 5 + 15/2 \times 3,5 \right] + 84/2 \times 90 = 32 235 \text{ m}^3$$

$$\text{Total remoção} = 50 715 \text{ m}^3$$

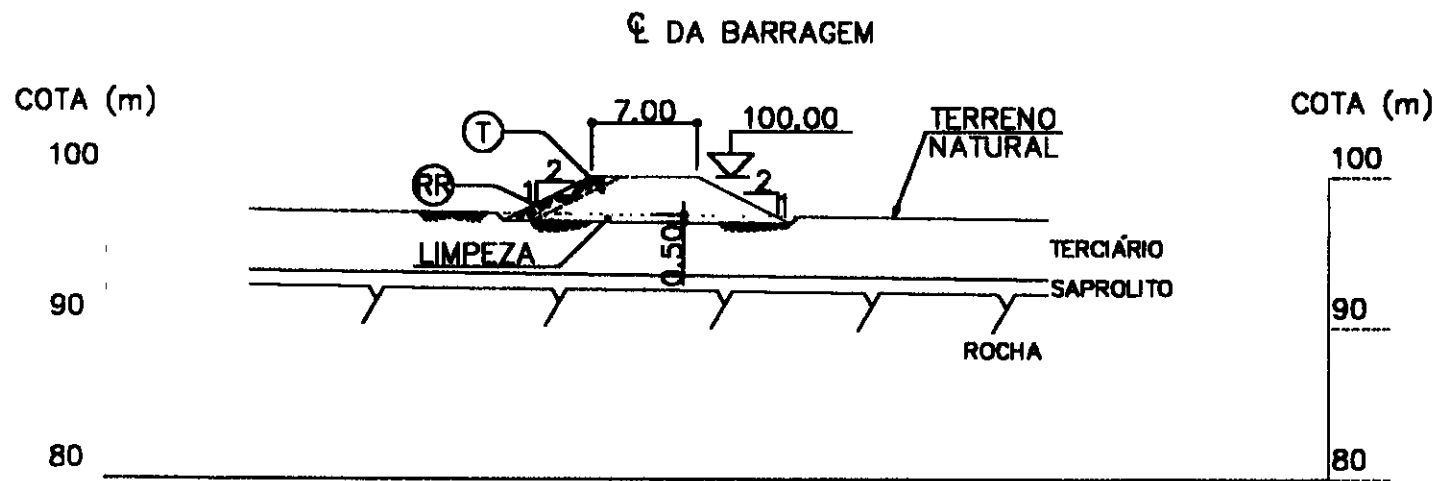
4. Preenchimento entre escadaria de montanha e a

Barragem

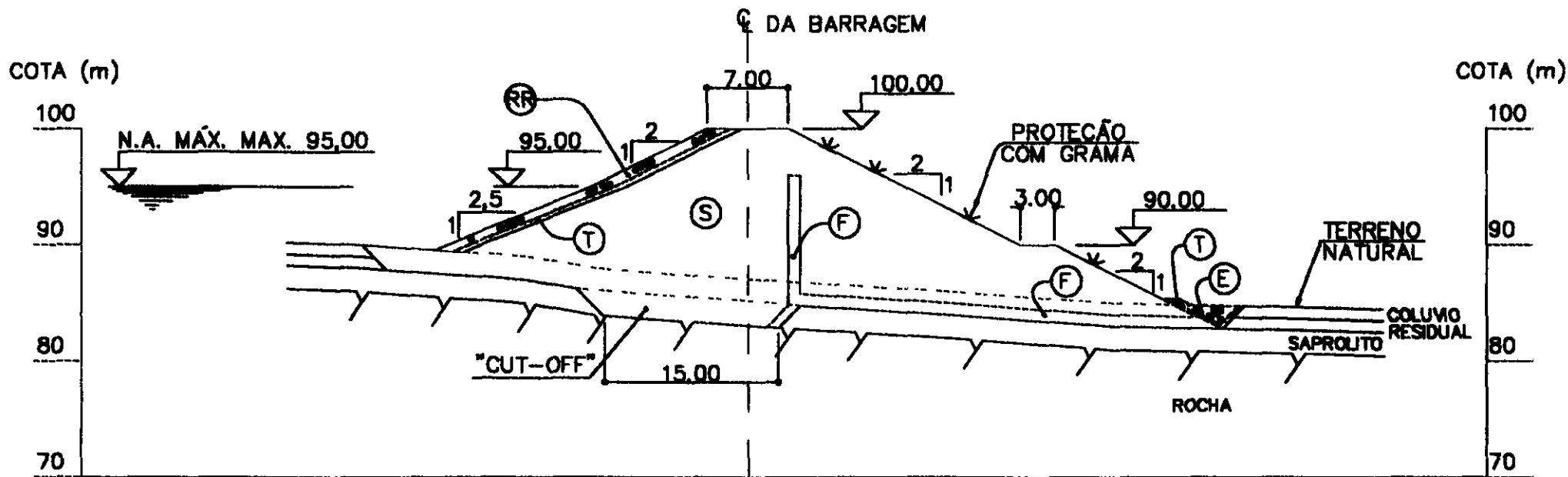
$$A = \left[ (29 + 7)/2 \times 6 \right] = 108 \text{ m}^2$$

$$V = \left[ 108 \times 105 \right] + \left[ 108/2 \times 65 \right] = \underline{\underline{14 850 \text{ m}^3}}$$

$$\text{Volume solo escadaria} - 142 000 + 15 000 = \underline{\underline{157 000 \text{ m}^3}}$$



TÍPICA ENTRE ESTACAS 0+0,00 A 25+0,00

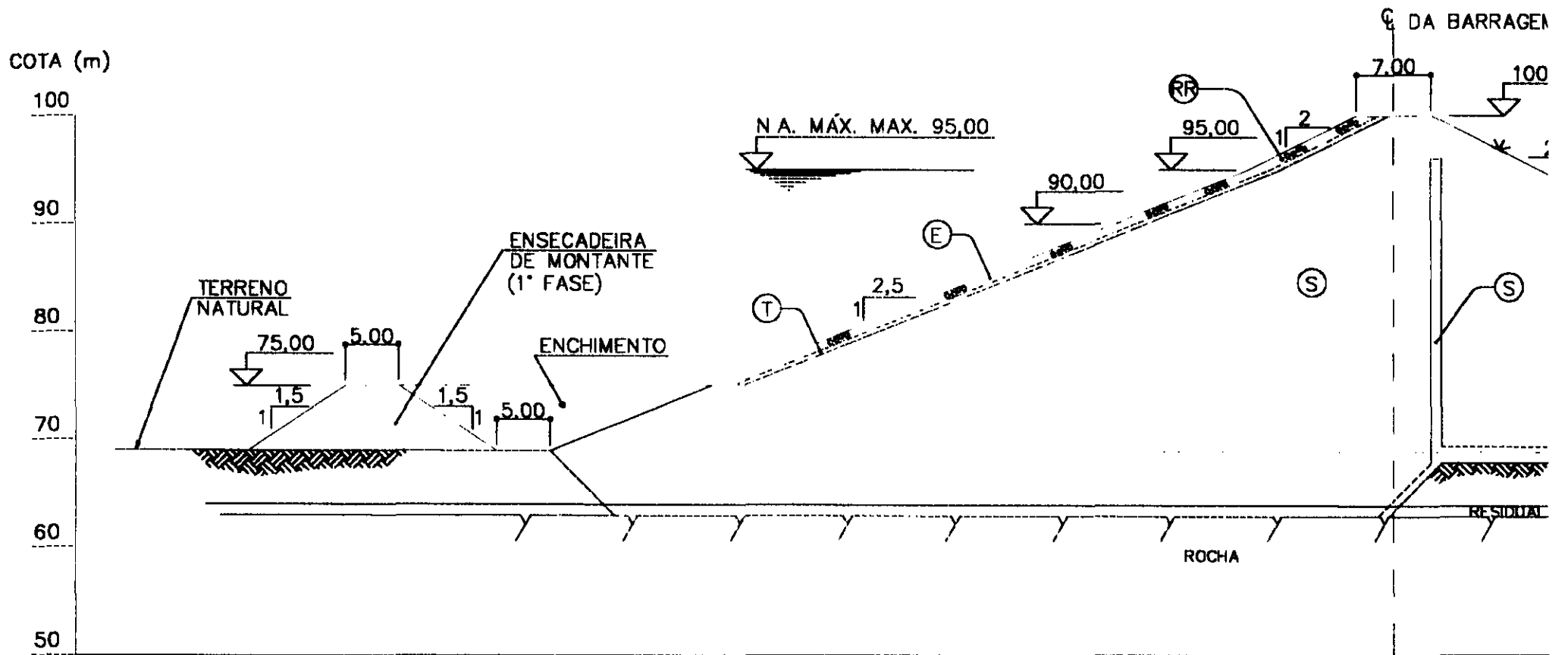


ESTACA 30+0,00

ESC. 1:500

TÍPICA ENTRE ESTACAS 25+0,00 A 37+0,00

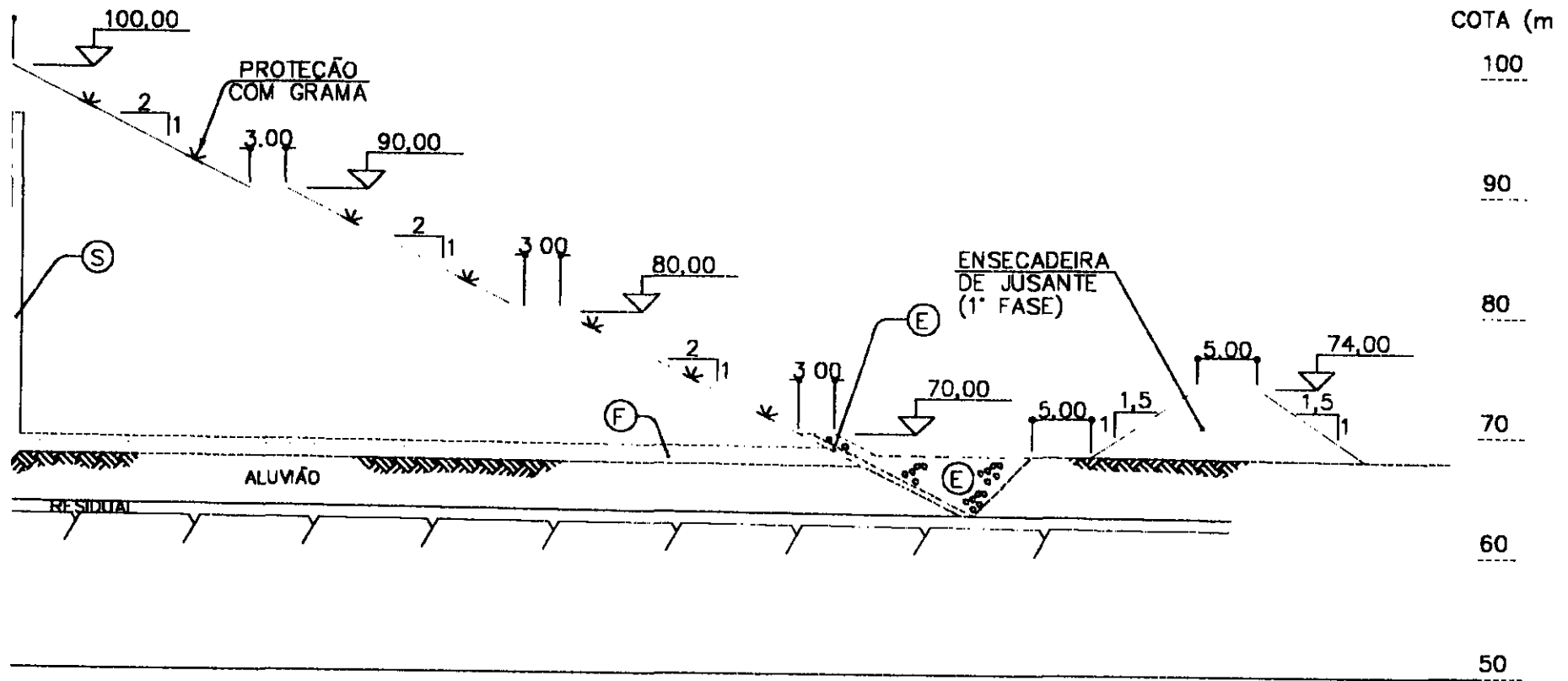
000030



ESTACA 45-0,0  
 ESC 1/500

TÍPICA ENTRE ESTACAS 37+0,0  
 000031

A BARRAGEM

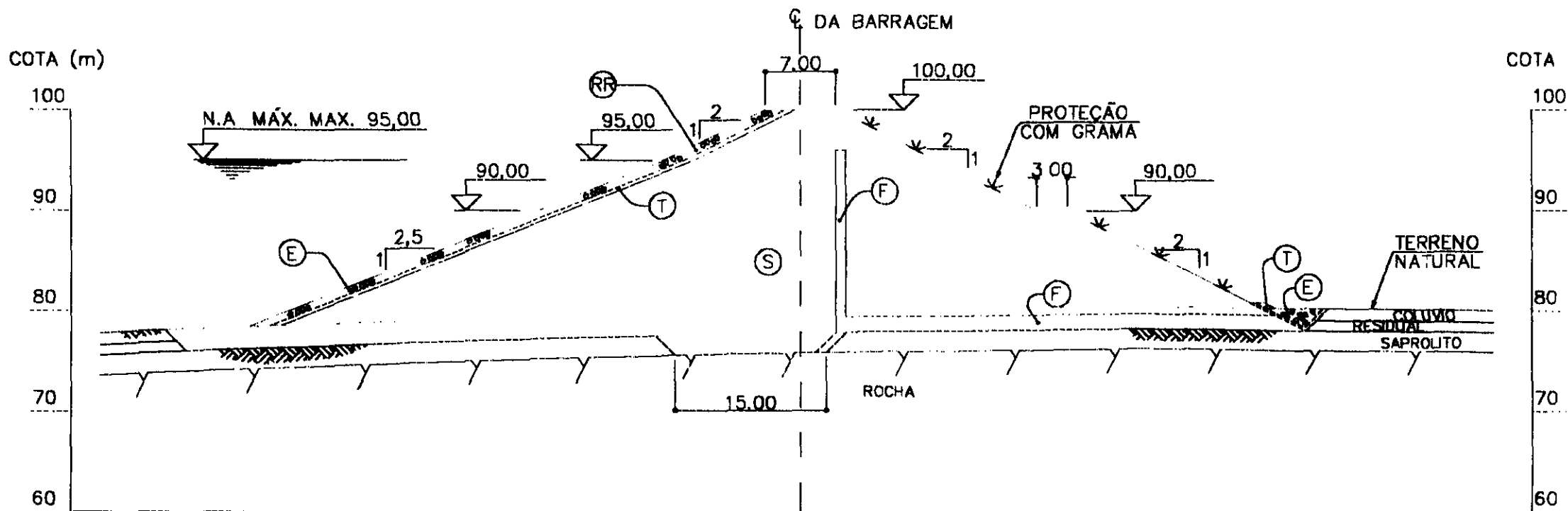


45 ± 0,00

S 37+0 00 A 53+0,00

000032

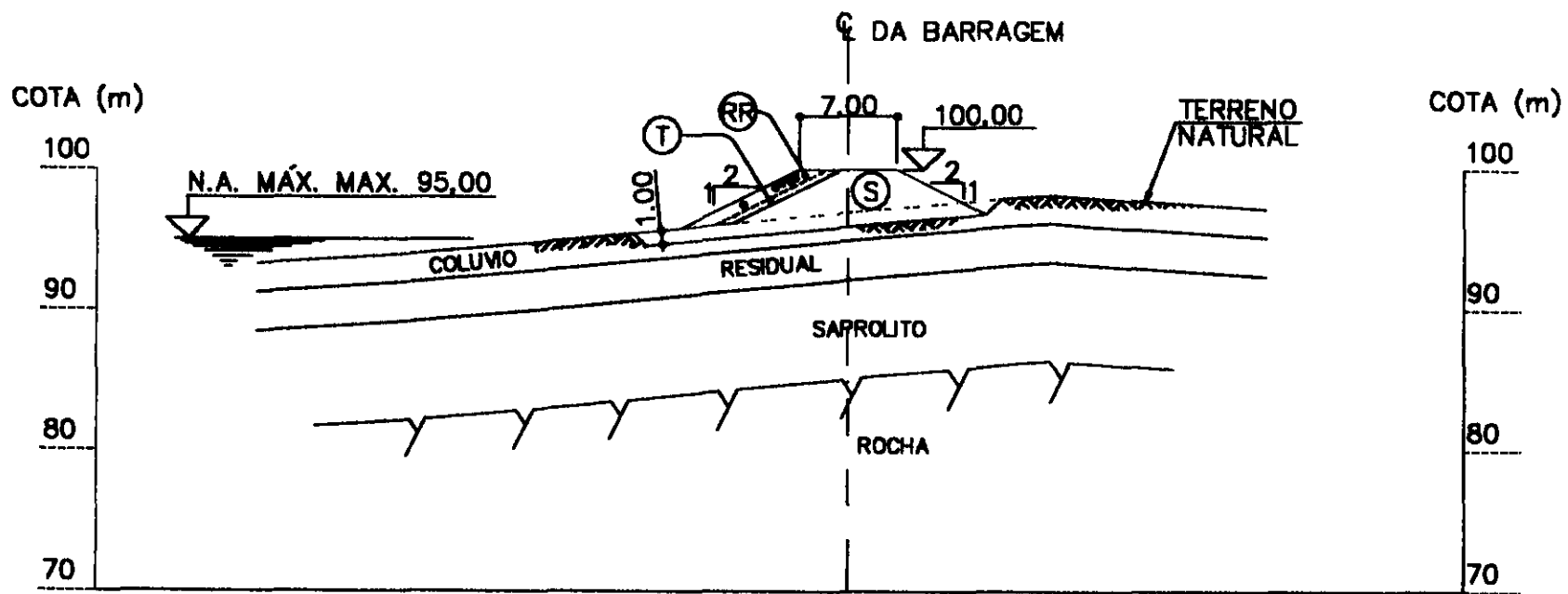




ESTACA 60+0,00  
 ESC 1/500

TÍPICA ENTRE ESTACAS 53+0,00 A 63+0,00

000033

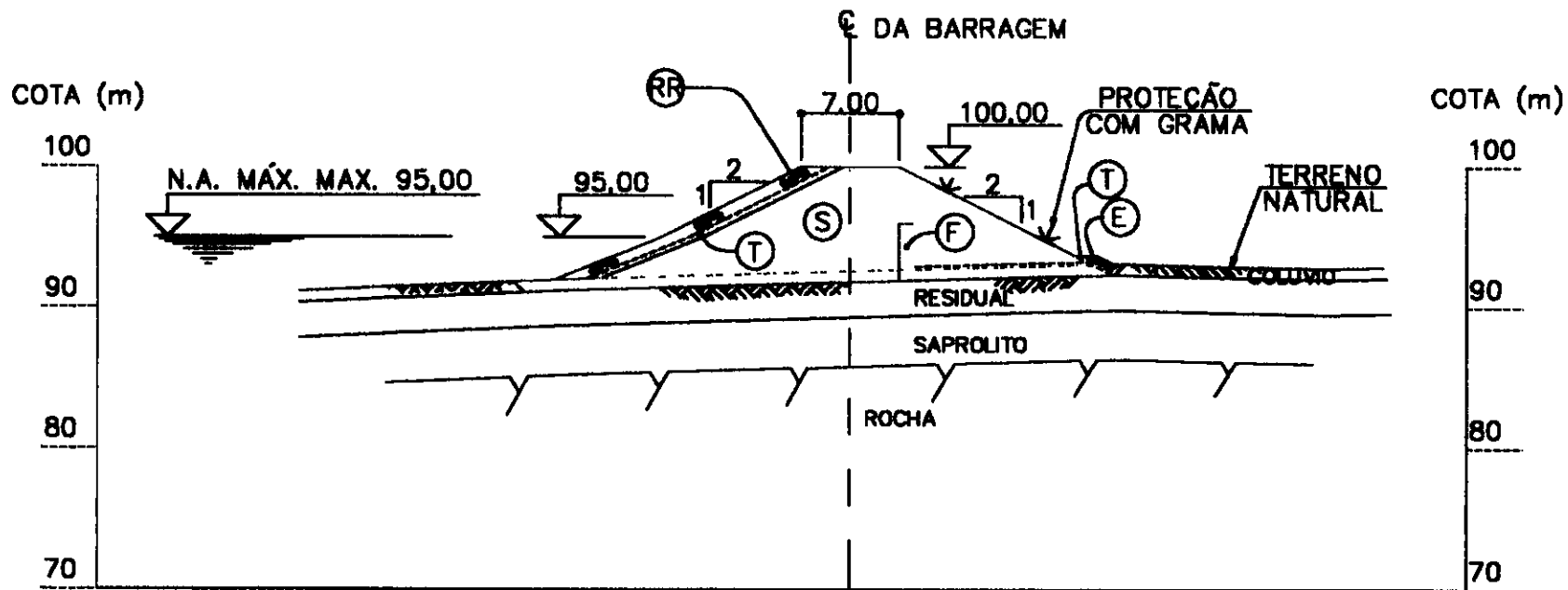


ESTACA 70+0,00

ESC. 1:500

TÍPICA ENTRE ESTACAS 63+0,00 A 73+0,00

000034



ESTACA 75+0,00  
 ESC. 1:500

TÍPICA ENTRE ESTACAS 73+0,00 A 90+0,00

000035

# ANTEPROJETO

## QUANTITATIVOS DAS OBRAS DE TERRA

1. Dreno de Pé - BLR até cota 70,00 +  
 BME -  $l \approx 200$  m  
 BLR (T) 3.600  
 (E) 10.500

BME (T) 500  
 (E) 1.200

### 2. Filtros

Secc.	A	A	anica	anica
			0	
	E	E	43,17	1,59
	C	C	142,73	59,97
	D	D	59,97	15
	E	E		
	F	F	10,95	
	G	G	8,15	

$$\begin{aligned}
 \text{Acc. A} &= \left[ \frac{43,17}{2} \times 30 \right] + \left[ \frac{(43,17 + 142,73)}{2} \times 300 \right] + \\
 & \left[ \frac{(142,73 + 59,97)}{2} \times 300 \right] + \left[ \frac{59,97}{2} \times 170 \right] + \left[ \frac{10,95}{2} \times 100 \right] \\
 & + \left[ \frac{(10,95 + 15)}{2} \times 150 \right] + \left[ \frac{15 + 8,15}{2} \times 150 \right] + \left[ \frac{8,15}{2} \times 200 \right]
 \end{aligned}$$

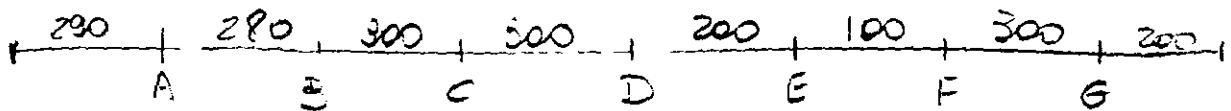
$$(A) \quad 69.080 \text{ m}^3$$

$$\begin{aligned}
 \text{Bute} &= \left[ \frac{1,59}{2} \times 30 \right] + \left[ \frac{(1,59 + 33)}{2} \times 300 \right] + \left[ \frac{(33 + 1,59)}{2} \times 300 \right] \\
 & \left[ \frac{1,59}{2} \times 170 \right] = 10.536 \text{ m}^3
 \end{aligned}$$

### 3. CASCALHO

Seção A - A	-	3,65 m <sup>2</sup>
B - B	-	22,12 m <sup>2</sup>
C - C	-	48,34 m <sup>2</sup>
D - D	-	27,76 m <sup>2</sup>
E - E	-	3,86
F - F	-	9,22
G - G	-	8,45

valores retornado  
no CAD



$$\begin{aligned} & \textcircled{C} \left( \frac{3,65}{2} \times 290 \right) + \left[ \frac{3,65 + 22,12}{2} \times 280 \right] + \left[ \frac{22,12 + 48,34}{2} \times 300 \right] \\ & + \left[ \frac{48,34 + 27,76}{2} \times 300 \right] + \left[ \frac{27,76 + 3,86}{2} \times 200 \right] + \left[ \frac{3,86 + 9,22}{2} \times 100 \right] \\ & + \left[ \frac{9,22 + 8,45}{2} \times 300 \right] + \left[ \frac{8,45}{2} \times 200 \right] = 33\,432 \text{ m}^2 \end{aligned}$$

$$\textcircled{C} \quad 33\,500 \text{ m}^2$$

### 4. TOTAIS

Ⓢ	alternativa	1	=	1.310.700
ⓕ	"	"	=	86.100
Ⓣ	"	"	=	21.400
ⓇⓇ	"	"	=	25.700
ⓔ	"	"	=	18.200

PARA ANTEPROJETO

- (S)  $1.310.700 - 33.500 = 1.277.200 \text{ m}^3$
- (C)  $33.500 \text{ m}^3$
- (F<sub>A</sub>)  $69.100 \text{ m}^3$
- (F<sub>B</sub>)  $10.600 \text{ m}^3$
- (T)  $21.400 + 4.100 = 25.500 \text{ m}^3$
- (RR)  $25.700 \text{ m}^3$
- (E)  $18.200 + 11.700 = 29.900 \text{ m}^3$

# ANTE PROJETO

## ENSECADEIRAS

1. MD

cota 69 - 84 m<sup>2</sup>

$$V_{MD} = [84/2 \times 190] + [84 \times 40] + [84 \times 400] = 44940 \\ + (84 \times 20) = 46620$$

2. LR

cota 67 - 318,5 m<sup>2</sup> cota 80

cota 67 - 180 cota 78

cota 69 - 236,5 - cota 80

cota 69 - 166,5 cota 78

$$V_{ME_M} = [236,5/2 \times 150] + [236,5 + 318,5/2 \times 50] + [236,5 \times 80] \\ + [62,5 \times 50] = 53.657 \text{ m}^3$$

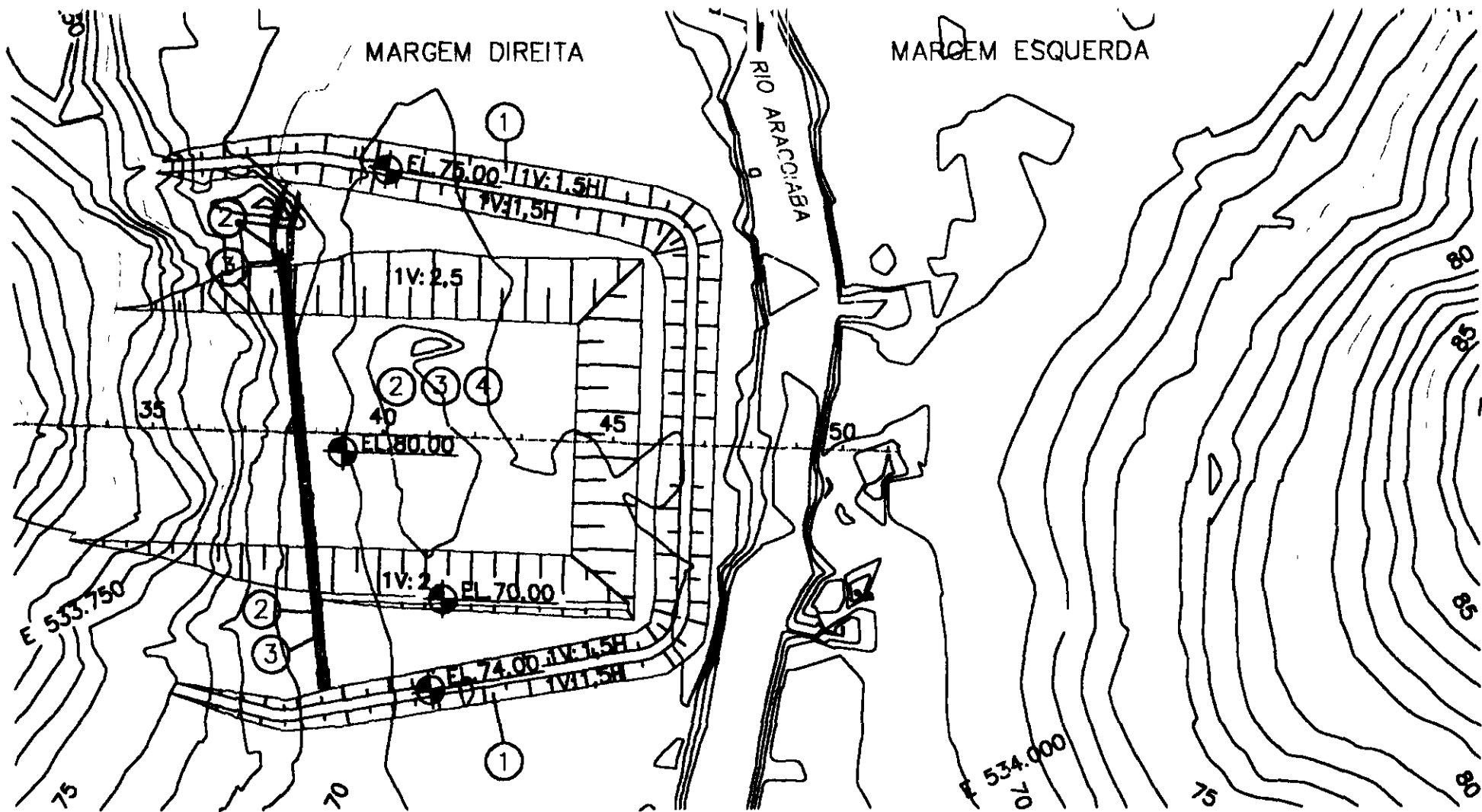
$$V_{ME_J} = [166,5/2 \times 130] + [166,5 \times 120] + [166,5 + 180/2 \times 40] \\ + [166,5 \times 20] = 41.063 \text{ m}^3$$

$$\text{TOTAL} = 141.340 \text{ m}^3 + 3000 = 54340$$

Remoção -

$$\textcircled{5} 84 \times 170 = 14.280 \text{ m}^3 \rightarrow 14300 \text{ m}^3$$

Preenchimento - 18000 m<sup>3</sup>

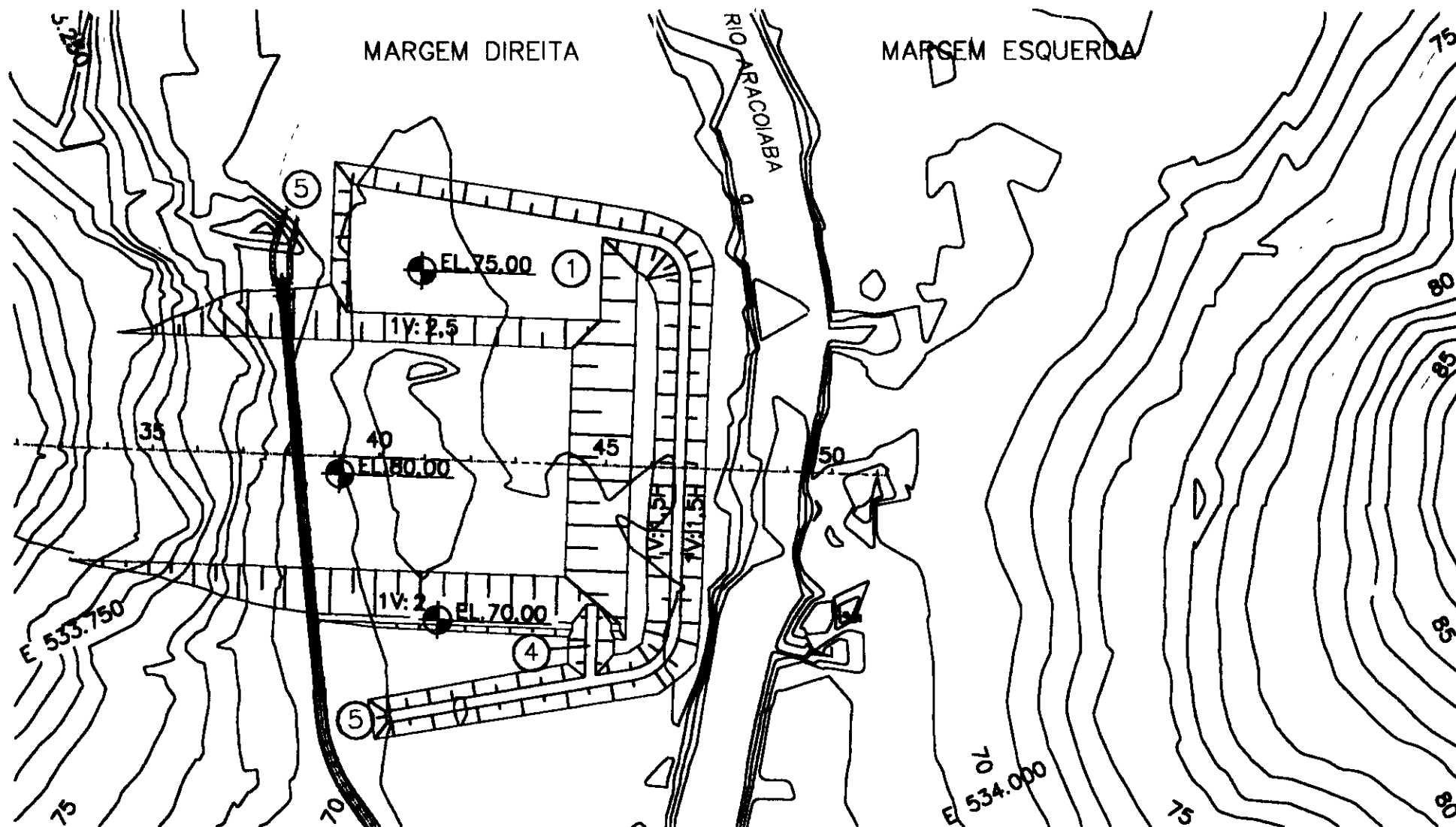


FASE 1A

- 1- CONSTRUÇÃO DA ENSECADORA, COM MATERIAL DAS ESCAVAÇÕES DA BARRAGEM, EM PONTA DE ATERRO OU EM CAMADAS, ATÉ A ELEVAÇÃO 75,00 A MONTANTE E 74,00 A JUSANTE.
- 2- ESCAVAÇÕES NA FUNDAÇÃO DA BARRAGEM, DA GALERIA DE DESVIO E DO VERTEDOIRO
- 3- TRATAMENTO DA FUNDAÇÃO DA BARRAGEM E CONCRETAGEM DA GALERIA DE DESVIO.
- 4- EXECUÇÃO DA BARRAGEM NO RECINTO ENSECADO, ATÉ A COTA 80,00 E CONCRETAGEM DO VERTEDOIRO

000040

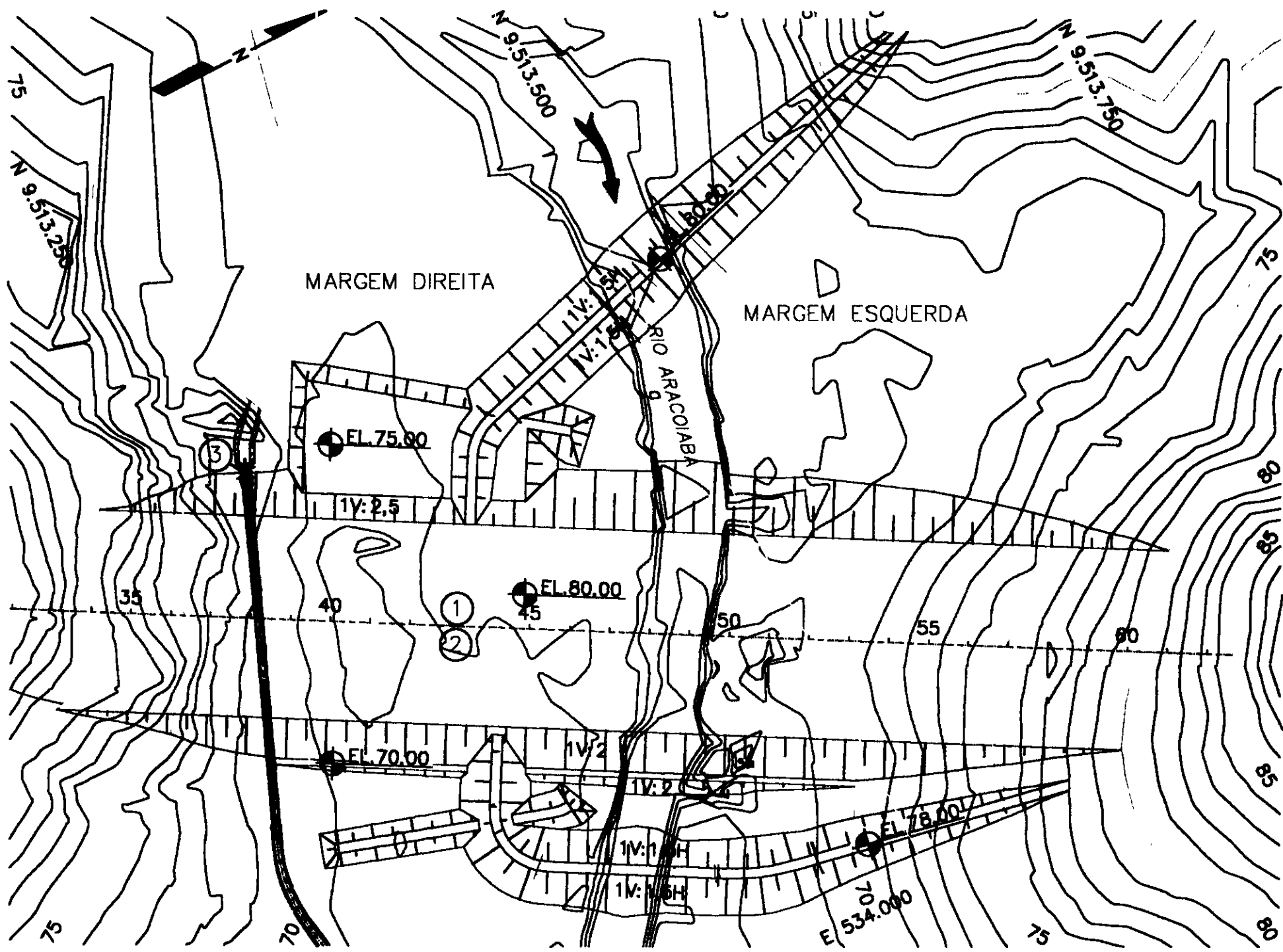




FASE 1B

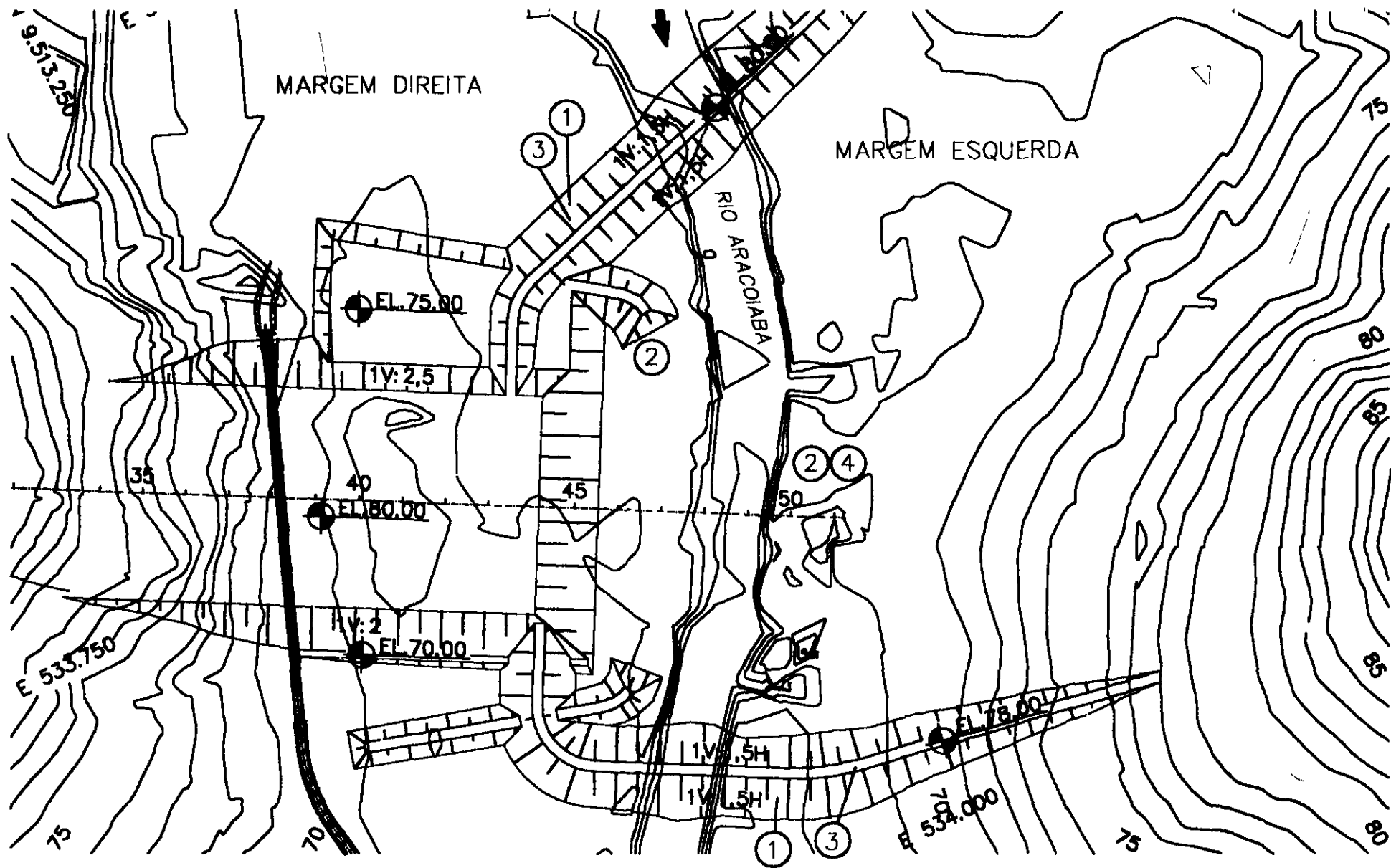
- 1- PREENCHIMENTO ATÉ A COTA 75,00 DO ESPAÇO ENTRE A ENSECADEIRA DE MONTANTE E BARRAGEM
- 2- CONSTRUÇÃO DA BARRAGEM A DIREITA DO VERTEDOIRO.
- 3- CONSTRUÇÃO DA BARRAGEM NA MARGEM ESQUERDA.
- 4- CONSTRUÇÃO DA ENSECADEIRA DE LIGAÇÃO ENTRE BARRAGEM E ENSECADEIRA DE JUSANTE.
- 5- REMOÇÃO DA ENSECADEIRA NA ENTRADA E SAÍDA DA GALERIA DE DESVIO.

000041



FASE 2B

000042



FASE 2A

- 1- DESVIO DO RIO PARA A GALERIA ATRAVÉS DA CONSTRUÇÃO DAS ENSECADREIAS DE MONTANTE E JUSANTE NA COTA 75,00.
- 2- REMOÇÃO DA ENSECADREIRA TRANSVERSAL DA ETAPA 1, ESCAVAÇÃO DA FUNDAÇÃO DA BARRAGEM.
- 3- ALTEAMENTO DA ENSECADREIRA DE MONTANTE PARA A COTA 80,00 E JUSANTE PARA A COTA 78,00.
- 4- TRATAMENTO DA FUNDAÇÃO DA BARRAGEM NA REGIÃO DO LEITO DO RIO.

---

**4 - QUANTITATIVOS DAS ESTRUTURAS HIDRÁULICAS**

Os quantitativos para as obras hidráulicas tiveram como base os seguintes desenhos de projeto

- AR-ET-4001 - Estruturas Hidráulicas - Alternativa 1 - Planta e Cortes
- AR-ET-4002 - Estruturas Hidráulicas - Alternativa 2 - Planta e Cortes
- AR-ET-4003 - Estruturas Hidráulicas - Alternativa 3 - Planta e Cortes
- AR-ET-4004 - Estruturas Hidráulicas - Alternativa 4 - Planta e Cortes
- AR-ET-4004 - Estruturas Hidráulicas - Galeria de Desvio - Alternativas 1, 2 e 3

Foram calculados os volumes das estruturas e de escavação de acordo com a memória de cálculo apresentada a seguir

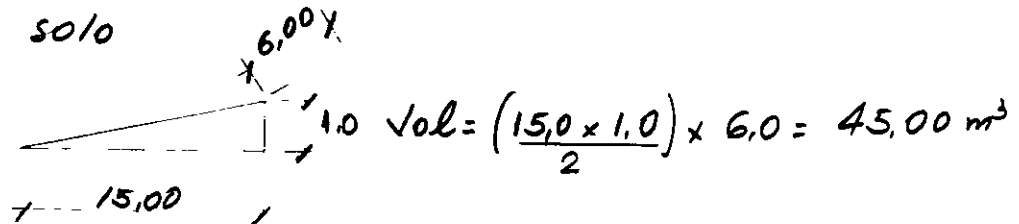
# BARRAGEM ARACOÁBA

## GALERIA DE DESVIO

### 1- ESCAVAÇÃO

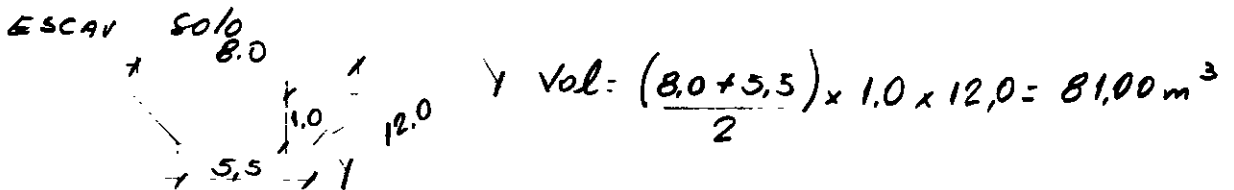
#### 1.1- CANAL DE APROXIMAÇÃO

ESCAV. SOLO

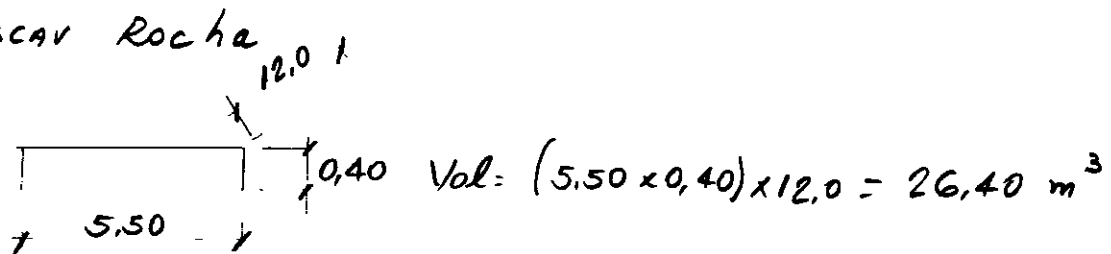


#### 1.2- TOMADA D'ÁGUA

ESCAV. SOLO

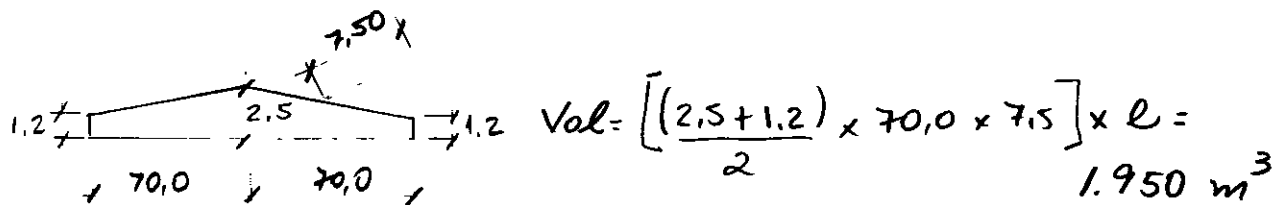


ESCAV. Rocha

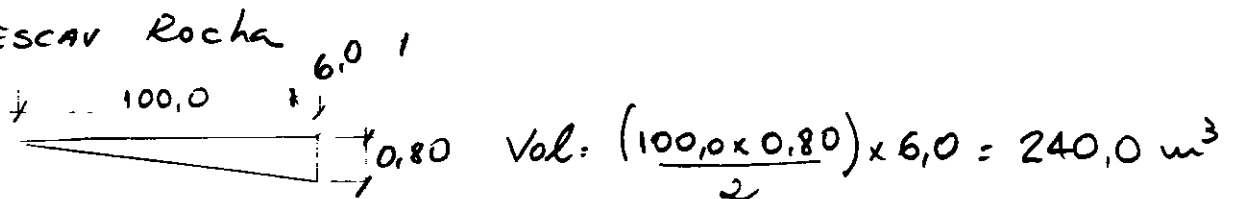


#### 1.3- TÚNEL

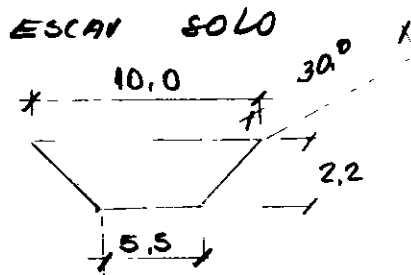
ESCAV. SOLO



ESCAV. Rocha

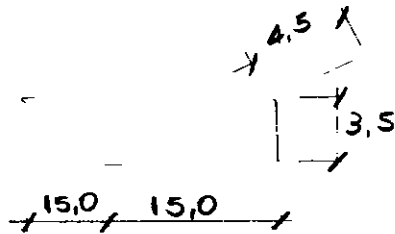


## 1.4 - BACIA DE DISSIPACÃO



$$\text{Vol} = \left( \frac{10,0 + 5,5}{2} \right) \times 2,20 \times 30,0 = 515,0 \text{ m}^3$$

ESCAVAÇÃO ROCHA

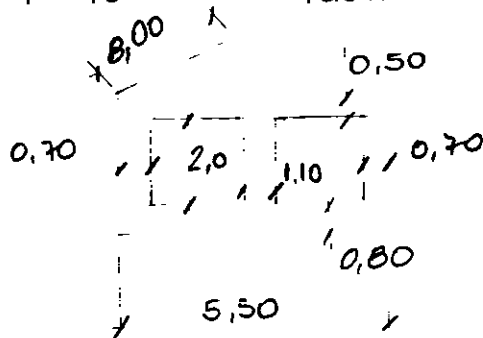


$$\text{Vol} = \left( \frac{15,0 \times 3,5}{2} \right) \times 4,50 = 120,0 \text{ m}^3$$

$$(15,0 \times 3,5) \times 4,50 = 236,0 \text{ m}^3$$

## 2 - CONCRETO

### 2.1 - TOMADA D'ÁGUA



$$\text{ÁREA} = 5,50 \times 0,50 = 2,75 \text{ m}^2$$

$$5,50 \times 0,80 = 4,40 \text{ m}^2$$

$$2,00 \times 0,70 = 1,40 \text{ m}^2$$

$$2,00 \times 0,70 = 1,40 \text{ m}^2$$

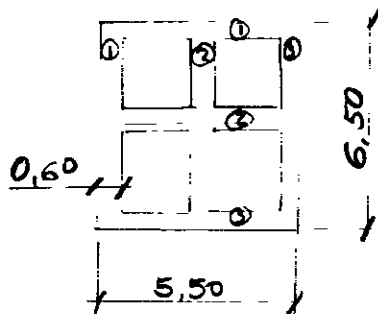
$$2,00 \times 1,10 = 2,20 \text{ m}^2$$

$$\underline{12,15 \text{ m}^2}$$

$$\text{Vol} = 12,15 \text{ m}^2 \times 8,00 = 97,20 \text{ m}^3$$

### 2.2 - TORRE DA TOMADA D'ÁGUA

$$h = 27,0 \text{ m}$$

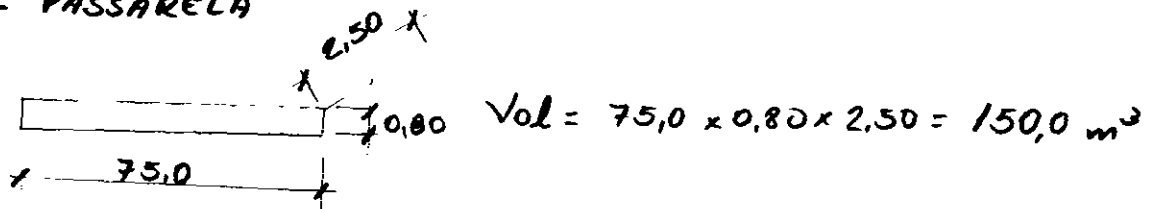


$$[(6,50 \times 0,60) \times 3] \times 27,0 = 315,90 \text{ m}^3$$

$$[(5,50 \times 0,60) \times 3] \times 27,0 = 267,30 \text{ m}^3$$

$$\underline{583,20 \text{ m}^3}$$

### 2.3 - PASSARELA



$$\text{Vol} = 75,0 \times 0,80 \times 2,50 = 150,0 \text{ m}^3$$

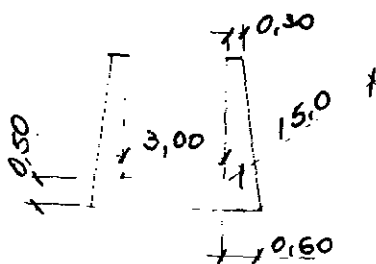
### 2.4 - TÚNEL

ÁREA DA SEÇÃO =  $8,55 \text{ m}^2$  (MEDIDO NO CAD)

COMPRIMENTO DO TÚNEL = 140,0 m

$$\text{Vol} = 140,0 \times 8,55 = 1200,0 \text{ m}^3 - 350,00 \text{ m}^3 = 850 \text{ m}^3$$

### 2.5 - BACIA DE DISSIPACÃO



$$\text{Vol. com s/forma} = 5,0 \times 140 \times 0,50 = 350$$

$$h_{\text{médio}} = 5,225 \text{ m}$$

$$\text{Vol} = \left[ \frac{(0,60 + 0,30)}{2} \times 5,225 \right] \times 15,0 \times 2 = 70,50 \text{ m}^3$$

$$(3,00 \times 0,5) \times 15,0 = 22,50 \text{ m}^3$$

$$h = 7,50 \text{ m}$$

$$\text{Vol} = \left[ \frac{(0,60 + 0,30)}{2} \times 7,50 \right] \times 15,0 \times 2 = 101,0 \text{ m}^3$$

$$(3,00 \times 0,5) \times 15,0 = 22,50 \text{ m}^3$$

$$\text{total} = 216,50 \text{ m}^3$$

LIMPEZA E TRATAMENTO DE FUNDAÇÃO EM ROCHA

COMPR - 220,0 m

larg - 5,0 m

$$\text{ÁREA} = 1.100 \text{ m}^2$$

### RESUMO CONCRETO

CONCRETO c/FORMA 1700 m<sup>3</sup>

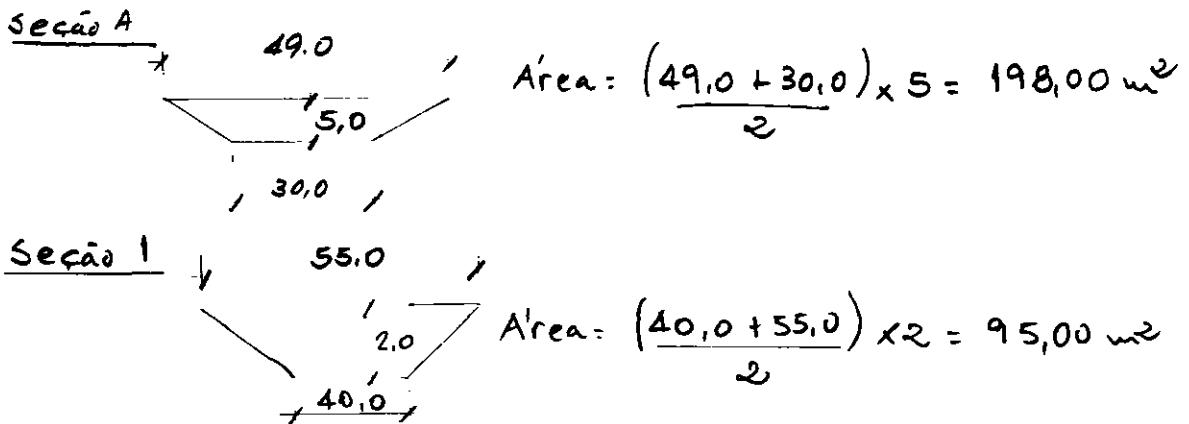
s/FORMA 410 m<sup>3</sup>

ESTRUTURAS HIDRÁULICAS - ALTERNATIVA 1

1- ESCAVAÇÃO

1.1 - CANAL DE APROXIMAÇÃO

ESCAV. SOLO

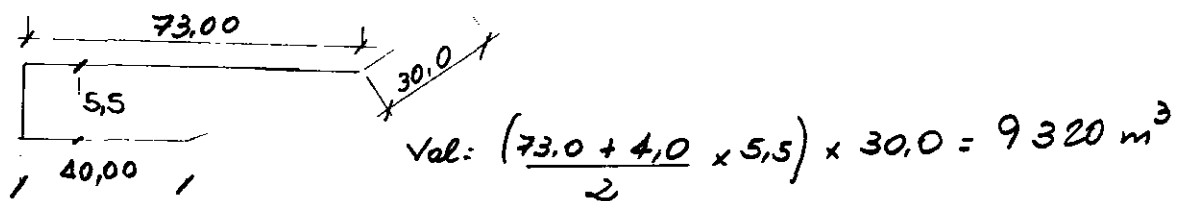


Volume

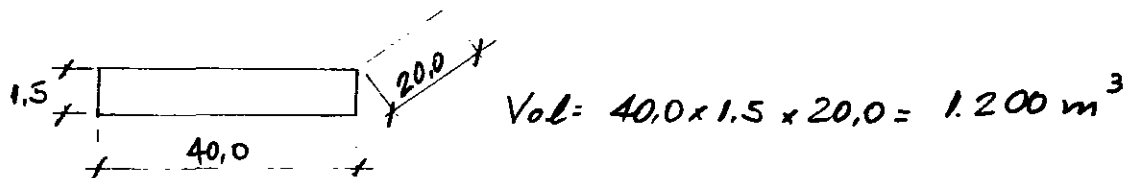
Seção	Área	Área/2	DIST	Vol
0	0			
1	95,0	47,50	40,0	1 900
A	198,0	146,50	70,0	10 255
A'	198,0	198,00	20,0	3 960
				<u>16 115 m<sup>3</sup></u>

1.2 - VERTEDOURO

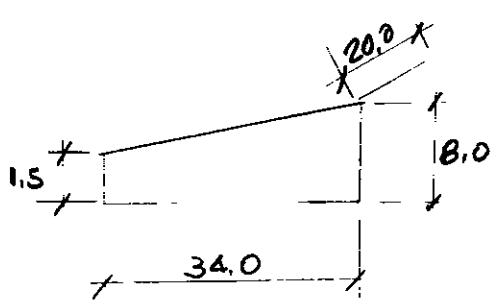
ESCAV SOLO



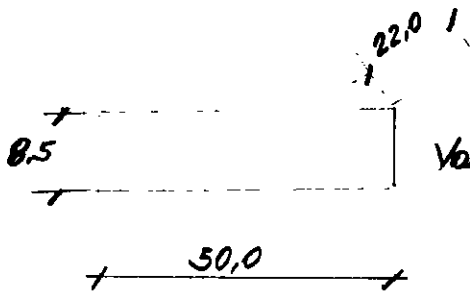
ESCAV Rocha







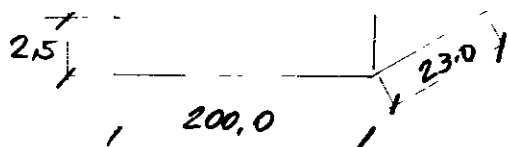
$$\text{Vol.} = \frac{(8,0 + 1,5 \times 34,0)}{2} \times 20,0 = 3\,230 \text{ m}^3$$



$$\text{Vol.} = (50,0 \times 8,5) \times 22,0 = 9\,350 \text{ m}^3$$

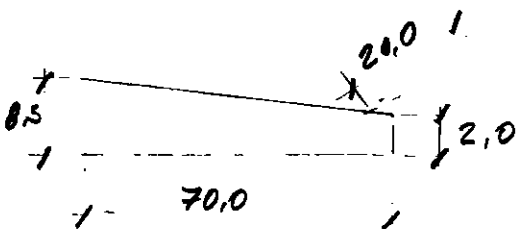
### 1.3 - CANAL DE RESTITUIÇÃO

ESCAV. EM SOLO

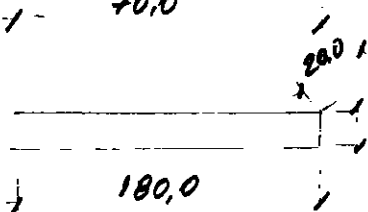


$$\text{Vol.} = (200,0 \times 2,5 \times 23,0) = 11.500 \text{ m}^3$$

ESCAV EM ROCHA



$$\text{Vol.} = \frac{(8,5 + 2,0)}{2} \times 70,0 \times 20,0 = 7\,350 \text{ m}^3$$



$$\text{Vol.} = (180,0 \times 2,0) \times 20,0 = 7.200 \text{ m}^3$$

RESUMO

ESCAV Solo = 36.935 m<sup>3</sup>

ESCAV Rocha = 28 330 m<sup>3</sup>

DESMATAMENTO = 20.000 m<sup>2</sup>

TRAT Rocha = 8.000 m<sup>2</sup>

TRAT. Solo = 6.300 m<sup>2</sup>

## 2- CONCRETO

### 21- VERTEDOURO

#### LAJE MONTANTE

$$15,0 \times 0,50 \times 20,0 = 150,0 \text{ m}^3$$

#### MUROS MONTANTE (2x)

$$x = 0,30$$

$$\text{COMPR} = 32,0 \text{ m}$$

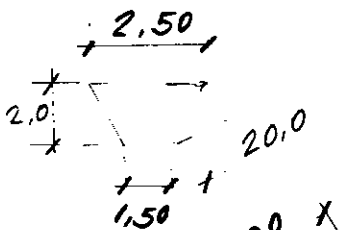
$$h = 6,0$$

$$\text{ÁREA} = \frac{0,30 + 0,50}{2} \times 6 = 2,4 \text{ m}^2$$

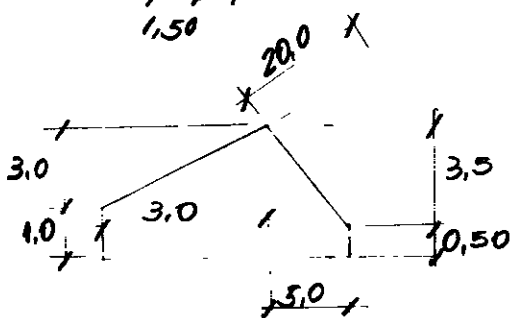
$$\text{Vol} = (2,4 \text{ m}^2 \times 32,0 \text{ m}) \times 2 = \underline{153,60 \text{ m}^3}$$



#### VERTEDOURO



$$\left( \frac{2,50 + 1,50}{2} \times 2,0 \right) \times 20,0 = 80,0 \text{ m}^3$$

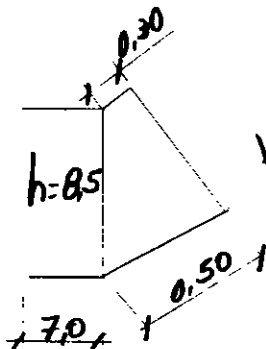


$$\text{ÁREA} = \frac{(4,0 + 1,0)}{2} \times 3,0 = 7,50 \text{ m}^2$$

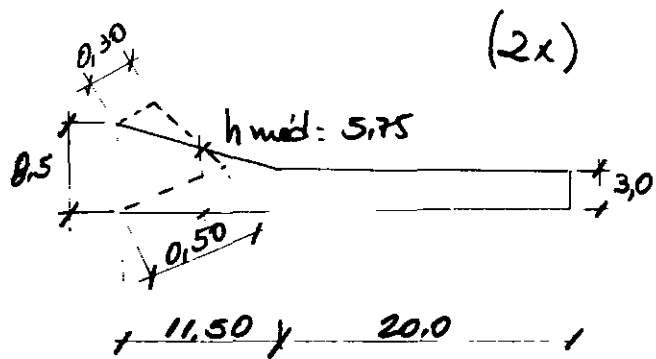
$$\text{ÁREA} = \frac{(4,0 + 0,5)}{2} \times 5,0 = 11,25 \text{ m}^2$$

$$\text{Vol} = (7,50 \text{ m}^2 + 11,25 \text{ m}^2) \times 20,0 = 375,0 \text{ m}^3$$

#### MUROS VERTEDOURO



$$\text{Vol} = \left( \frac{0,30 + 0,50}{2} \right) \times 0,85 \times 7,0 \times 2 = 23,80 \text{ m}^3 \times 2 = 47,60 \text{ m}^3$$



$$\text{Vol} = \frac{(0,30 + 0,50)}{2} \times 5,75 \times 11,50 = 26,45 \text{ m}^3$$

$$\text{Vol} = \frac{(0,30 + 0,50)}{2} \times 3,0 \times 20,0 = 24,00 \text{ m}^3$$

$$\begin{aligned} \text{total} &= 50,45 \text{ m}^3 \\ &\quad \times 2 \\ \hline &100,90 \text{ m}^3 \end{aligned}$$

### LAME JUSANTE

$$\text{Vol} = 20,0 \times 100,00 \times 0,50 = 1.000,0 \text{ m}^3$$

### CONCRETO PROJETADO (ESP. 0,10)

Diagram showing a rectangular cross-section with a width of 70,0 and a height of 3,0.

$$\text{ÁREA} = 70,0 \times 3,0 = 210,0 \text{ m}^2 \times 2 =$$

$$\underline{420,0 \text{ m}^2}$$

### RESUMO

$$\text{CONCRETO c/FORMA} - 775 \text{ m}^3$$

$$\text{S/FORMA} - 1.150 \text{ m}^3$$

$$\text{CONC. Projetado} - 420 \text{ m}^2$$

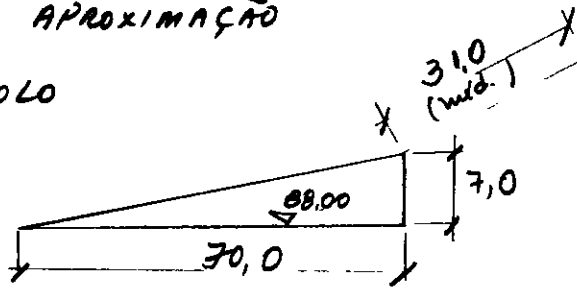
ESTRUTURAS HIDRÁULICAS

ALTERNATIVA 2

1 - ESCAVAÇÃO

1.1 - CANAL DE APROXIMAÇÃO

ESCAV. SOLO



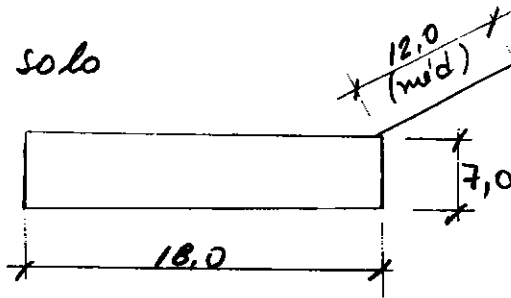
$$\text{Vol: } \frac{70,0 \times 7,0}{2} \times 31,0 = 7595 \text{ m}^3 \checkmark$$

TRATAMENTO EM SOLO

$$70,0 \times 22,0 = 1.540 \text{ m}^2 \checkmark$$

1.2 - VERTEDOURO

ESCAV. SOLO



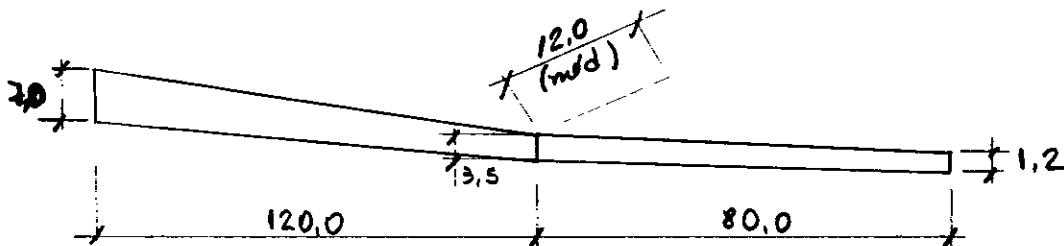
$$\text{Vol: } 18,0 \times 7,0 \times 12,0 = 1512,0 \text{ m}^3 \checkmark$$

TRATAMENTO EM SOLO

$$18,0 \times 12,0 = 216,0 \text{ m}^2 \checkmark$$

1.3 - CANAL - RÁPIDO

ESCAV EM SOLO



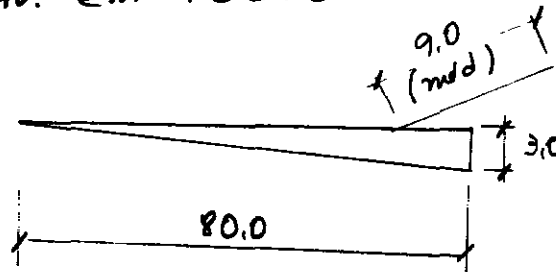
$$\text{Vol} = \left[ \frac{(7,0 + 3,5)}{2} \times 120,0 \right] \times 12,0 + \left[ \frac{(3,5 + 1,2)}{2} \times 80,0 \right] \times 12,0 = 7560 + 2256 = 9816,0 \text{ m}^3 \checkmark$$

TRATAMENTO SOLO

$$120,0 \times 8,0 = 960,0 \text{ m}^2 \checkmark$$

000052

ESCAV. EM ROCHA

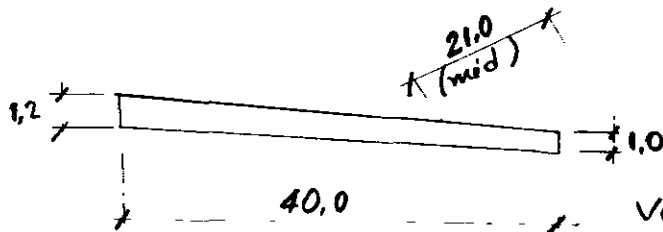


$$\text{Vol} = \frac{(80,0 \times 3,0) \times 9,0}{2} = 1.080 \text{ m}^3$$

$$\frac{\text{LIMPEZA E TRATAMENTO (Rocha)}}{80,0 \times 9,0 = 720 \text{ m}^2}$$

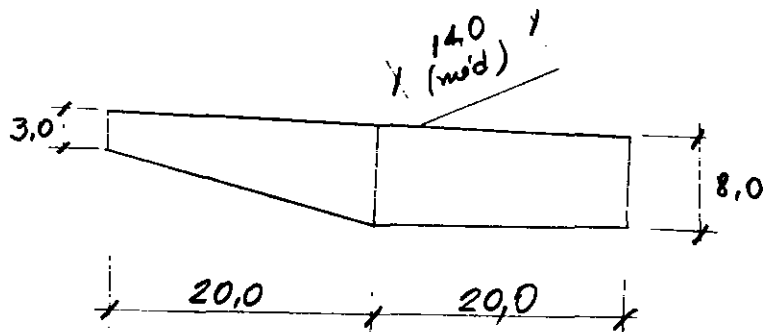
1.4 - BACIA DE DISSIPAÇÃO

ESCAV EM SOLO



$$\text{Vol} = \left[ \frac{(1,2 + 1,0) \times 40}{2} \right] \times 21,0 = 924,0 \text{ m}^3$$

ESCAV EM ROCHA



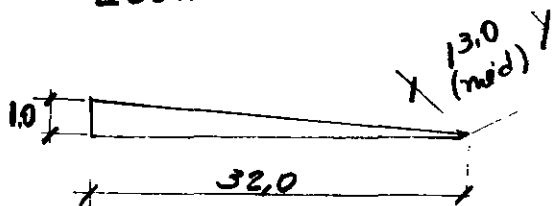
$$\text{Vol} = \left[ \frac{(3,0 + 8,0) \times 20,0}{2} \right] \times 14,0 + (20,0 \times 8,0) \times 14,0 = 1.540,0 + 2.240,0 = 3.780,0 \text{ m}^3$$

LIMPEZA E TRATAMENTO (ROCHA)

$$40,0 \times 14,0 = 560,0 \text{ m}^2 \checkmark$$

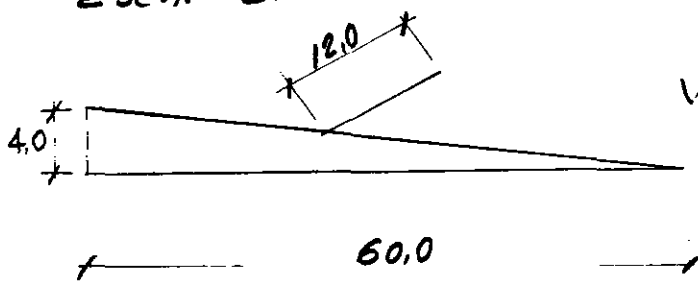
1.5 - CANAL DE RESTITUIÇÃO

ESCAV EM SOLO



$$\text{Vol} = \frac{(32,0 \times 1,0) \times 13,0}{2} = 208,0 \text{ m}^3 \checkmark$$

ESCAR EM ROCHA



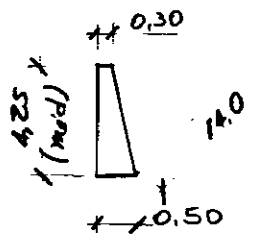
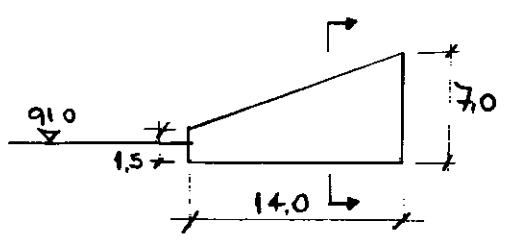
$$Vol = \left( \frac{60,0 \times 4,0}{2} \right) \times 12,0 = 1.440,0 \text{ m}^3$$

LIMPEZA E TRATAMENTO (ROCHA)  
 $(60,0 \times 12,0) = 720,0 \text{ m}^3 \checkmark$

2 - CONCRETO ESTRUTURAL

2.1 - CANAL DE APROXIMAÇÃO

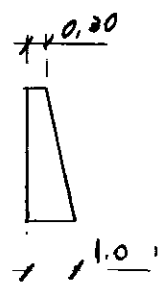
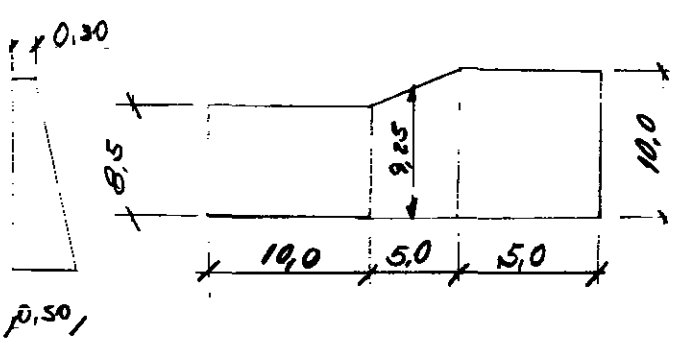
MURO LATERAL ESQUERDO



$$Vol = \left[ \left( \frac{0,30 + 0,50}{2} \right) \times 4,25 \right] \times 14,0 = 23,80 \text{ m}^3 \checkmark$$

2.2 - VERTEDOURO

MUROS LATERAIS



$$Vol = \left[ \left( \frac{0,30 + 0,50}{2} \right) \times 8,5 \right] \times 10,0 = 34,0 \text{ m}^3$$

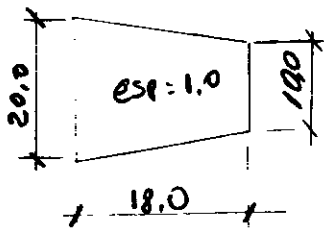
$$Vol = \left[ \left( \frac{0,30 + 0,50}{2} \right) \times 9,25 \right] \times 5,0 = 24,5 \text{ m}^3$$

$$Vol = \left[ \left( \frac{0,30 + 1,00}{2} \right) \times 10,0 \right] \times 5,0 = 32,5 \text{ m}^3$$

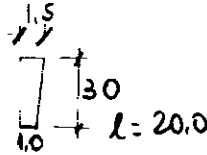
$$\leq 91,0 \text{ m}^3$$

0.11054 total =  $\frac{\times 2}{182,0 \text{ m}^3}$

LAJE -

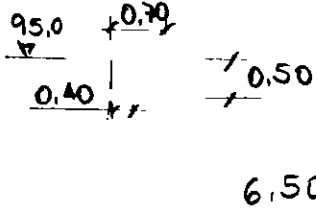


$$Vol = \left[ \left( \frac{20,0 + 18,0}{2} \right) \times 1,0 \right] \times 18,0 = 270,0 \text{ m}^3 \checkmark$$



$$Vol = \left( \frac{1,5 + 1,0}{2} \times 3,0 \right) \times 20,0 = 75,0 \text{ m}^3$$

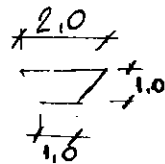
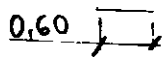
MURO DA SOLEIRA (comp = 24,5 m)



$$Vol = (0,70 \times 0,50 \times 24,5) = 8,6 \text{ m}^3$$

$$Vol = \left[ \left( \frac{0,40 + 0,60}{2} \right) \times 6,50 \right] \times 24,5 = 80,0 \text{ m}^3$$

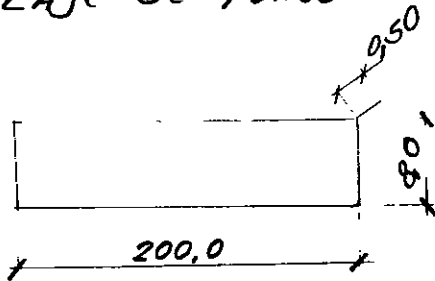
$$\text{total} = \underline{88,6 \text{ m}^3} \checkmark$$



$$Vol = \left( \frac{2,0 + 1,6}{2} \times 1,0 \right) \times 24,5 = \underline{37,0 \text{ m}^3} \checkmark$$

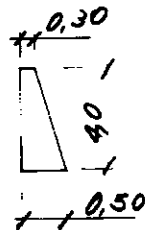
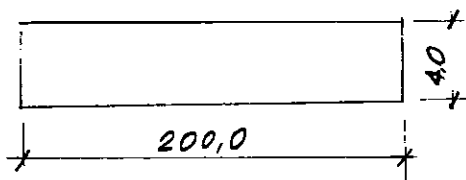
2.3 - CANAL - RÁPIDO

LAJE DE FUNDO



$$Vol = (200,0 \times 8,0 \times 0,50) = 800,0 \text{ m}^3 \checkmark$$

MUROS LATERAIS (2x)

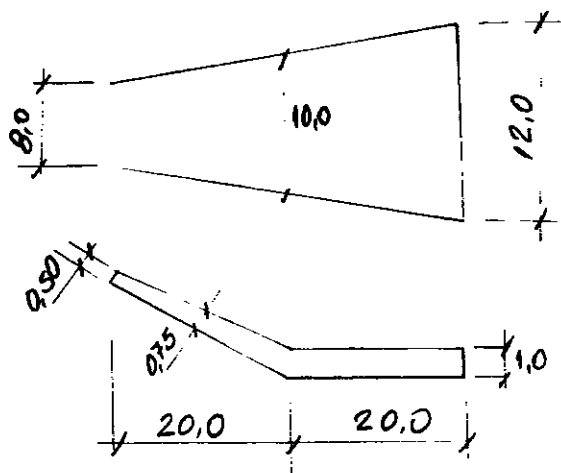


$$Vol = \left[ \left( \frac{0,30 + 0,50}{2} \right) \times 4,0 \right] \times 200,0 = 320,0 \text{ m}^3$$

$$\underline{\times 2} \\ 640,0 \text{ m}^3 \checkmark$$

2.4- BACIA DE DISSIPACÃO

LAJE DE FUNDO

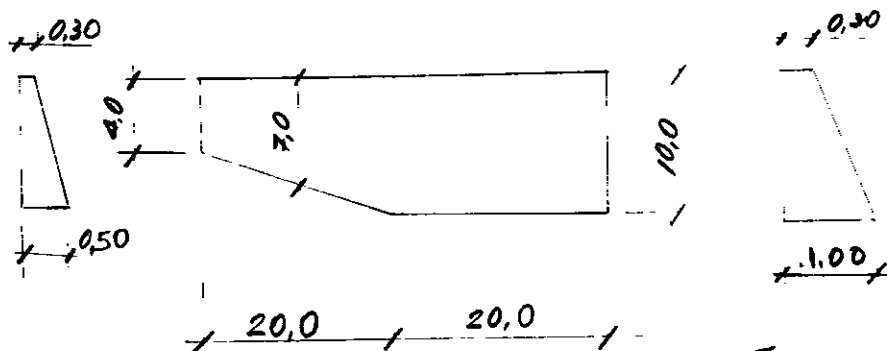


$$Vol = \left[ \left( \frac{8,0 + 10,0 \times 0,75}{2} \right) \right] \times 20,0 = 135,0 m^3$$

$$Vol = \left[ \left( \frac{10,0 + 12,0 \times 1,0}{2} \right) \right] \times 20,0 = 220,0 m^3$$

total = 355,0 m<sup>3</sup>

MUROS LATERAIS (2x)



$$Vol = \left[ \left( \frac{0,30 + 0,50}{2} \right) \times 7,0 \right] \times 20,0 = 56,0 m^3$$

$$Vol = \left[ \left( \frac{0,30 + 1,0}{2} \right) \times 10,0 \right] \times 20,0 = 130,0 m^3$$

total 186,00 m<sup>3</sup>

x 2  
372,00 m<sup>3</sup>

DESMATAMENTO E LIMPEZA

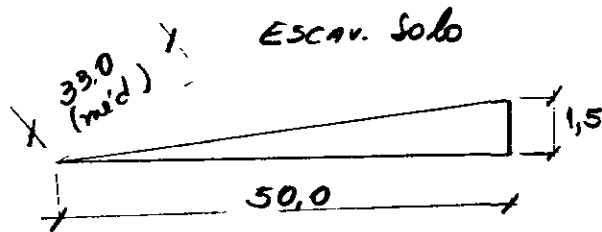
$$(30,0 \times 60,0) + (300,0 \times 25,0) = 9300 m^2$$



BARRAGEM ARACOIABAESTRUTURAS HIDRÁULICAS - ALTERNATIVA 3

## 1- ESCAVAÇÃO

## 1.1- CANAL DE APROXIMAÇÃO

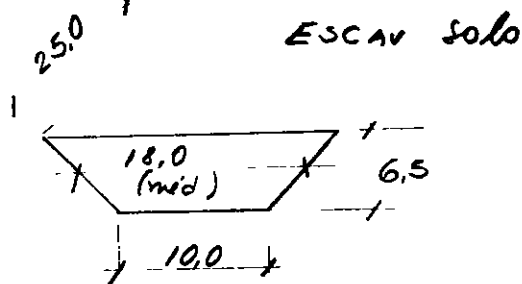


$$\text{Vol.} = \frac{50,0 \times 1,5 \times 33,0}{2} = \underline{1.237,5 \text{ m}^3} \checkmark$$

TRATAMENTO EM solo =

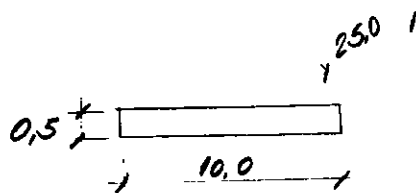
$$50,0 \times 33,0 = 1.650 \text{ m}^2 \checkmark$$

## 1.2- VERTEJOURO



$$\text{Vol.} = \frac{(18,0 + 10,0) \times 6,5}{2} \times 25,0 = \underline{2.275 \text{ m}^3} \checkmark$$

ESCAV Rocha



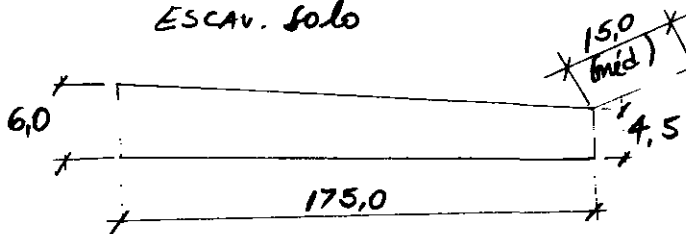
$$\text{Vol.} = 10,0 \times 0,5 \times 25,0 = \underline{125,0 \text{ m}^3} \checkmark$$

LIMPEZA E TRATAMENTO

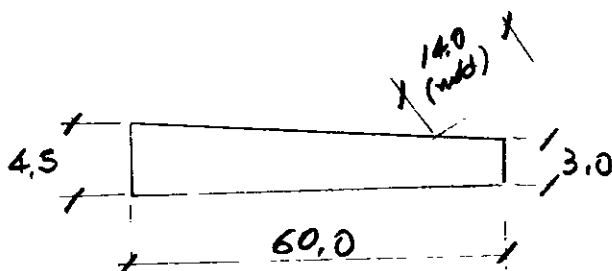
$$10,0 \times 25,0 = \underline{250,0 \text{ m}^2}$$

## 1.3 - CANAL - RÁPIDO

ESCAV. solo



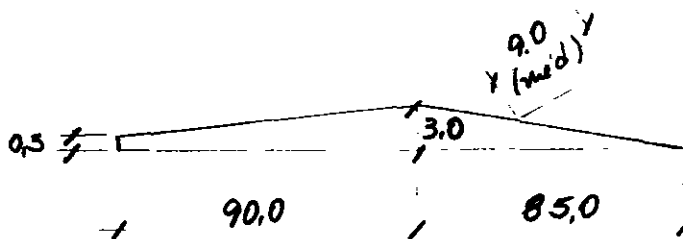
$$\text{Vol.} = \frac{(6,0 + 4,5) \times 175,0}{2} \times 15,0 = \underline{13.780,0 \text{ m}^3} \checkmark$$



$$\text{Vol.} = \frac{(4,5 + 3,0) \times 60,0}{2} \times 14,0 = \underline{3.150,0 \text{ m}^3}$$

$$\text{total} = \underline{16.930 \text{ m}^3} \checkmark$$

ESCAV. Rocha



$$Vol = \left[ \frac{0,5 + 3,0}{2} \times 90,0 \right] \times 9,0 = 1.417 m^3$$

$$Vol = \frac{3,0 \times 85,0}{2} \times 9,0 = 1.147 m^3$$

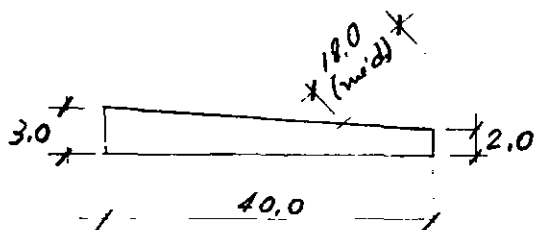
total: 2.564 m<sup>3</sup>

LIMPEZA E TRATAMENTO

175,0 x 9 = 1.575 m<sup>2</sup>

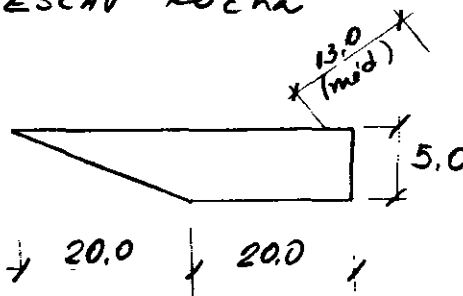
1.4 - BACIA DE DISSIPAÇÃO

ESCAV SOLO



$$Vol = \left[ \frac{3,0 + 2,0}{2} \times 40,0 \right] \times 18,0 = 1.800,0 m^3$$

ESCAV Rocha



$$Vol = \frac{5,0 \times 20,0}{2} \times 13,0 = 650,0 m^3$$

$$Vol = (5,0 \times 20,0) \times 13,0 = 1.300,0 m^3$$

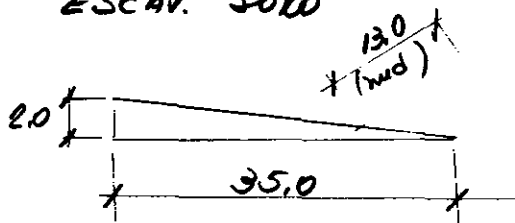
total = 1.950,0 m<sup>3</sup>

LIMPEZA E TRATAMENTO

40,0 x 13,0 = 520,0 m<sup>2</sup>

1.5 - CANAL DE RESTITUIÇÃO

ESCAV. SOLO

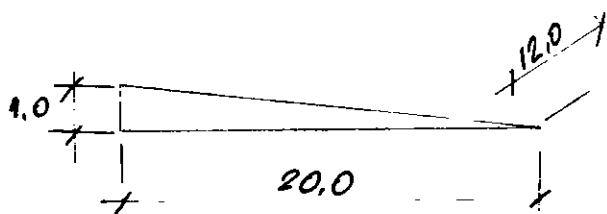


$$Vol = \frac{35,0 \times 2,0}{2} \times 13,0 = 455,0 m^3$$

TRATAMENTO EM SOLO

35,0 x 13,0 = 455 m<sup>2</sup>

## ESCAV EM ROCHA



$$\text{Vol} = \frac{(20,0 \times 1,0)}{2} \times 12,0 = \underline{120,0 \text{ m}^3}$$

## LIMPEZA E TRATAMENTO EM ROCHA

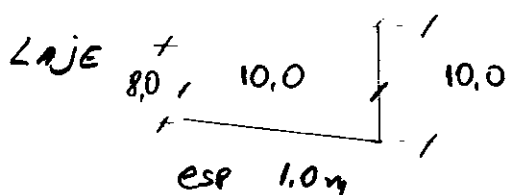
$$20,0 \times 12,0 = 240,0 \text{ m}^2$$

## DESMATAMENTO E LIMPEZA

$$(40 \times 60) + (150 \times 25) + (120 \times 30) = 9750 \text{ m}^2$$

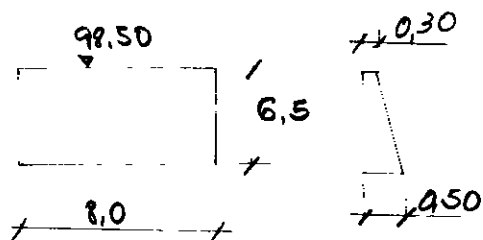
## 2 - CONCRETO ESTRUTURAL

### 2.1 - VERTEDOURO



$$\text{Vol} = \left[ \frac{(10,0 + 8,0)}{2} \times 10,0 \right] \times 1,0 = \underline{90,0 \text{ m}^3}$$

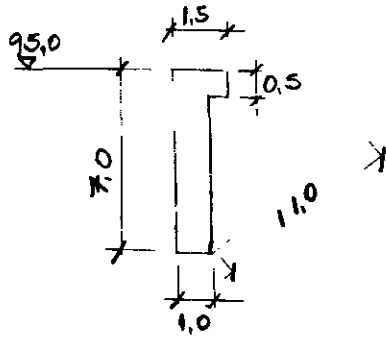
### MUROS LATERAIS (x 2)



$$\text{Vol} = \left[ \frac{(0,30 + 0,50)}{2} \times 6,5 \right] \times 8,0 = 20,80 \text{ m}^3$$

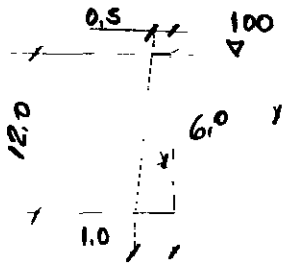
$$\text{total} = \frac{20,80 \times 2}{1} = \underline{41,60 \text{ m}^3}$$

MURO DA SOLEIRA



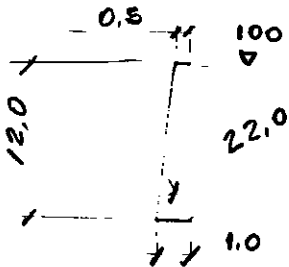
$$\text{Vol: } (6,5 \times 1,0 \times 11,0) + (1,5 \times 0,5 \times 11,0) = 71,5 \text{ m}^3 + 8,25 \text{ m}^3 = 80,0 \text{ m}^3 \checkmark$$

MURO LATERAL



$$\text{Vol: } \left( \frac{0,5 + 1,0}{2} \times 12,0 \right) \times 6,0 = 54,0 \text{ m}^3 \checkmark$$

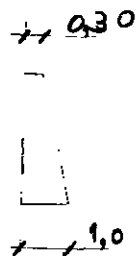
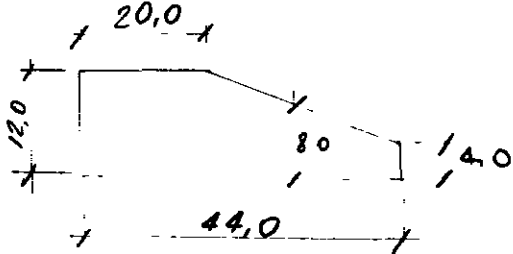
MURO DE FUNDO



$$\text{Vol: } \left( \frac{0,5 + 1,0}{2} \times 12,0 \right) \times 22,0 = 198,0 \text{ m}^3 \checkmark$$

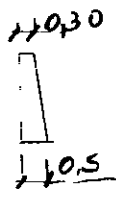
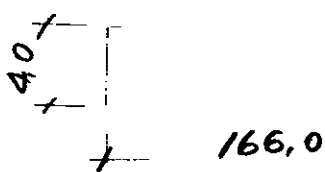
2 2 - CANAL - RÁPIDO

PAREDES (2x)



$$\text{Vol: } \left( \frac{0,3 + 0,5}{2} \times 12,0 \right) \times 20,0 = 96,0 \text{ m}^3$$

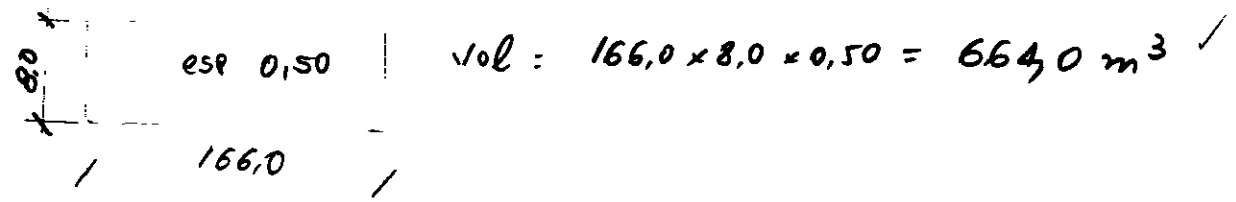
$$\text{Vol: } \left( \frac{0,3 + 0,5}{2} \times 8,0 \right) \times 24,0 = 76,8 \text{ m}^3$$



$$\text{Vol: } \left( \frac{0,3 + 0,5}{2} \times 4,0 \right) \times 166,0 = 265,6 \text{ m}^3$$

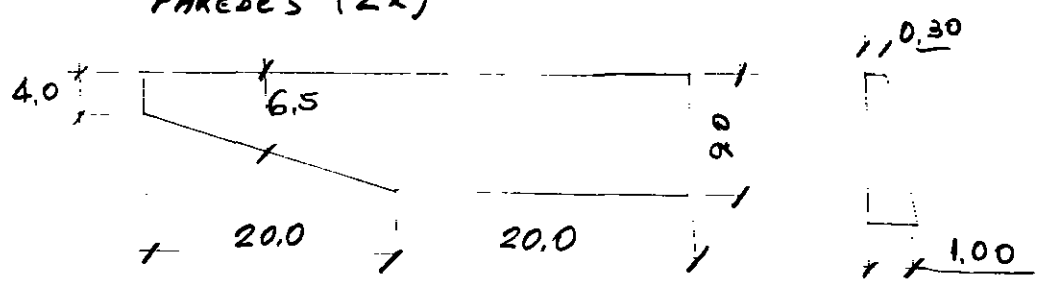
$$\text{total } 531,2 \text{ m}^3 \times 2 = 1.062,4 \text{ m}^3 \checkmark$$

LAJE



2.3 - BACIA DE DISSIPACÃO

PAREDES (2x)



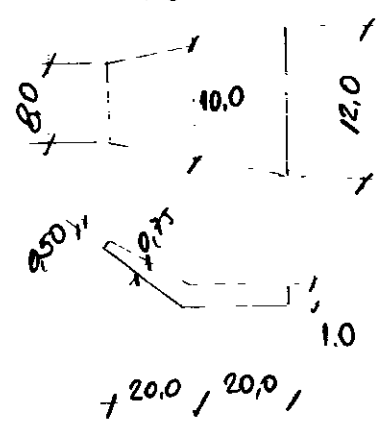
Vol:  $\frac{(0,3 + 0,5 \times 6,5)}{2} \times 20,0 = 52,0 \text{ m}^3$

Vol:  $\frac{(0,3 + 1,0 \times 9,0)}{2} \times 20,0 = 117,0 \text{ m}^3$

total: 169,0 m³

$\frac{169,0}{2} = 338,0 \text{ m}^3$  ✓

LAJE



Vol:  $\frac{(8,0 + 10,0 \times 0,75)}{2} \times 20,0 = 135,0 \text{ m}^3$

Vol:  $\frac{(10,0 + 12,0 \times 1,0)}{2} \times 20,0 = 220,0 \text{ m}^3$

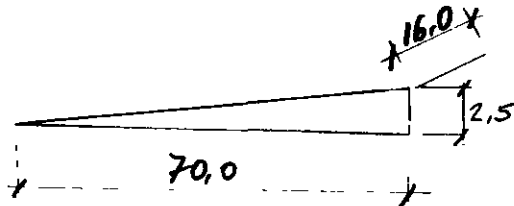
total: 355,0 m³ ✓

## BARRAGEM - ARACOIABA

ESTRUTURAS HIDRÁULICAS - ALTERNATIVA 4  
(TULIPA)

## 1- ESCAVAÇÃO

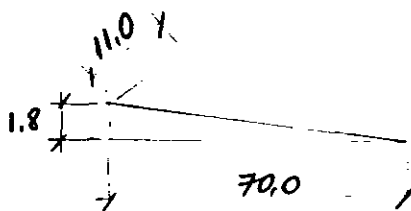
## 1.1- CANAL DE APROXIMAÇÃO



$$\text{Vol Solo} = \frac{70,0 \times 2,5 \times 16,0}{2} = 1.400 \text{ m}^3 /$$

$$\text{TRAT. Solo} = 70,0 \times 14,0 = 980,0 \text{ m}^2 /$$

## 1.2- CANAL DE RESTITUIÇÃO

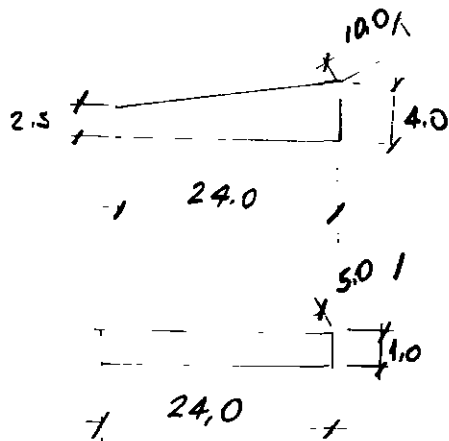


ESCA. SOLO

$$\text{Vol} = \frac{70,0 \times 1,8 \times 11,0}{2} = 693,00 \text{ m}^3$$

$$\text{TRAT. Rocha} = 70,0 \times 10,0 = 700 \text{ m}^2$$

## 1.3- TOMADA D'AGUA



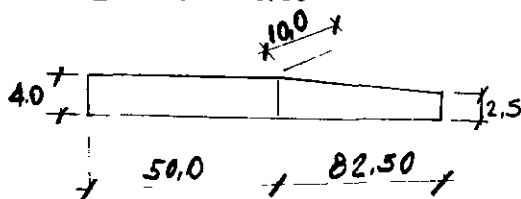
$$\text{ESCAV. Solo} = \left( \frac{2,5 + 4,0}{2} \times 24,0 \right) \times 10,0 = 780,0 \text{ m}^3 /$$

$$\text{ESCAV. Rocha} = (24,0 \times 1,0) \times 5,0 = 120,0 \text{ m}^3 /$$

$$\text{TRAT. Rocha} = 24,0 \times 5,0 = 120,0 \text{ m}^2$$

## 1.4 - GALERIA

ESCAV. Solo

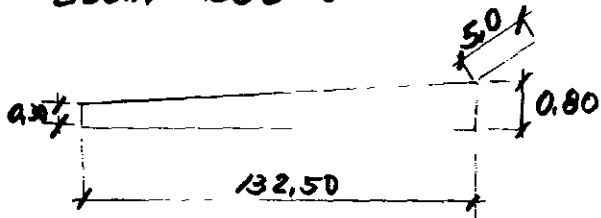


$$\text{Vol} = (50,0 \times 4,0) \times 10,0 = 2.000 \text{ m}^3$$

$$\left( \frac{4,0 + 2,5}{2} \right) \times 82,50 \times 10 = 2.680 \text{ m}^3$$

$$\text{TOTAL } 4.680 \text{ m}^3 /$$

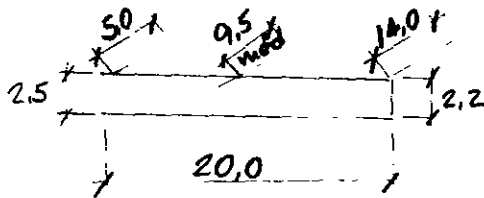
## ESCAV. Rocha



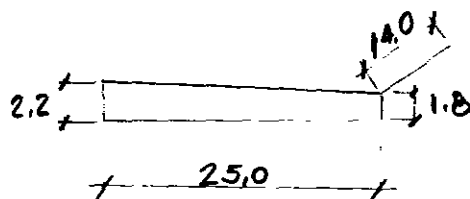
$$\text{Vol} = \left( \frac{0,20 + 0,80}{2} \right) \times 132,5 \times 5,0 = 365 \text{ m}^3$$

$$\text{TRAT Rocha} = 132,50 \times 5,0 = 662,50 \text{ m}^2$$

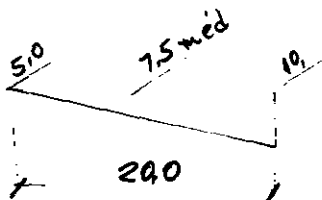
## 15 - BACIA DE DISSIPAÇÃO



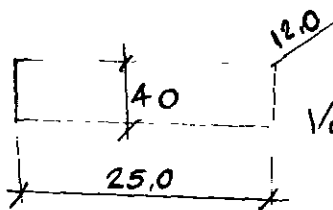
$$\text{Vol} : \text{Esc Solo} = \left( \frac{2,5 + 2,2}{2} \right) \times 20,0 \times 9,5 = 446 \text{ m}^3$$



$$\text{Vol} - \text{Esc Solo} = \left( \frac{2,2 + 1,8}{2} \right) \times 25,0 \times 14,0 = 700 \text{ m}^3$$



$$\text{Vol} - \text{Esc Rocha} = \left( \frac{20,0 \times 4,0}{2} \right) \times 7,5 = 300 \text{ m}^3$$



$$\text{Vol} - \text{Esc. Rocha} = \left( \frac{25,0 \times 4,0}{2} \right) \times 12,0 = 1.200 \text{ m}^3$$

$$\text{TRAT Rocha} = (7,5 \times 20,0) + (25,0 \times 12,0) = 450 \text{ m}^2$$

## RESUMO

$$\text{ESCAV Solo} = 8.700 \text{ m}^3$$

$$\text{ESCAV Rocha} = 1.700 \text{ m}^3$$

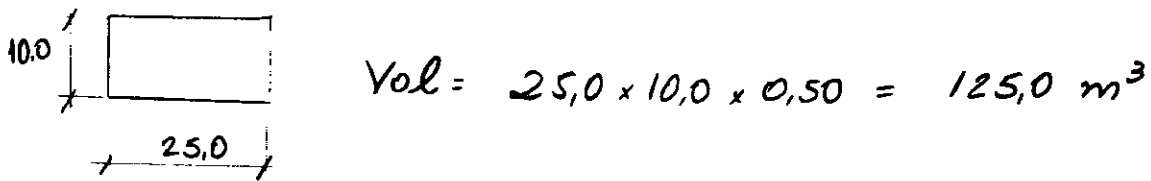
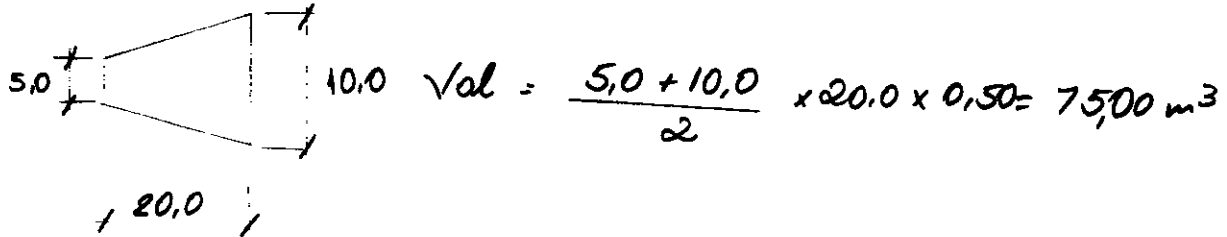
$$\text{TRAT Solo} = 980 \text{ m}^2$$

$$\text{TRAT. Rocha} = 1.950 \text{ m}^2$$

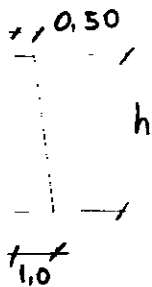
2 - CONCRETO

BACIA DE DISSIPACÃO

LAJE - ESP 0,50

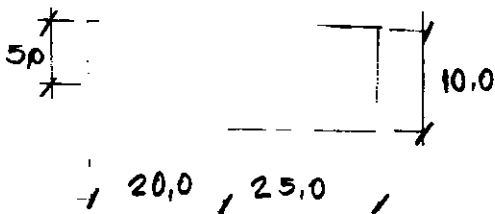


Muros (2x)



$$A_1 = \frac{0,5 + 1,0}{2} \times 10,0 = 7,5 \text{ m}^2$$

$$A_2 = \frac{0,5 + 0,75}{2} \times 5,0 = 3,13 \text{ m}^2$$



$$Vol = 7,5 \text{ m}^2 \times 20,0 \text{ m} = 150,0 \text{ m}^3$$

$$3,13 \text{ m}^2 \times 25,0 \text{ m} = 78,25 \text{ m}^3$$

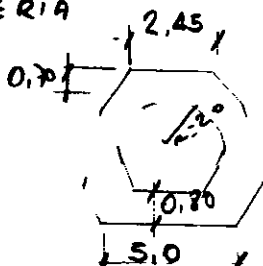
$$\underline{228,25 \text{ m}^3}$$

$$\times 2$$

$$\underline{\text{total} = 450 \text{ m}^3}$$

GALERIA

COM = 137m



$$ÁREA = 18,0 \text{ m}^2$$

$$Vol = 18,0 \text{ m}^2 \times 137,0 \text{ m} = 2466 \text{ m}^3$$



$$\text{TULIPA} = 64,50 \text{ m}^2 \times 20 \text{ m} = 1290 \text{ m}^3$$

$$\text{PASSARELA} = 130,00 \text{ m}^3$$

## RESUMO

$$\text{CONCRETO C/FORMA} = 4330,0 \text{ m}^3$$

$$\text{CONCRETO S/FORMA} = 200,0 \text{ m}^3$$

## 5 - ANÁLISES DE ESTABILIDADE

### 5.1 - Metodologia

A estabilidade dos taludes da barragem e de suas fundações foi verificada para a condição de equilíbrio limite, considerando superfícies de ruptura circulares, pelo método de Bishop Simplificado. Os cálculos foram efetuados em computador, utilizando o programa GSLOPE 3 32. Nesta fase, considerou-se a seção máxima da barragem, na baixada aluvionar junto à calha do rio (ver desenho AR-BT-3006 - Seções Típicas da Barragem - Seção na Estaca 45 + 0,00)

As análises foram feitas por tensões efetivas, com as poropressões no maciço determinadas segundo o seguinte critério

- Condição de "Final de Construção" - poropressões obtidas a partir de estimativas do valor do parâmetro " $r_u$ ",

- Condição de "Regime Permanente e Rebaixamento Rápido" - poropressões obtidas a partir de linhas piezométricas

Na condição de regime permanente considerou-se o nível d'água no reservatório à cota 95,00 m, correspondente ao máximo operacional, e uma piezométrica no maciço partindo da face do talude de montante nesta cota, com leve inclinação até atingir o filtro vertical de areia. Ao longo do tapete de areia até a saída, ao nível do rio, considerou-se um gradiente de 10%

Na condição de rebaixamento rápido manteve-se a mesma piezométrica dentro do maciço, prolongando-se para montante coincidente com a superfície do talude, até a cota da enseadeira (75,00 m)

Na fundação, as poropressões foram sempre referidas a uma linha piezométrica, para todas as condições de carregamento

Em todas as condições analisadas, considerou-se a ação de um sismo, representada por uma aceleração de 0,05g

### 5.2 - Fatores de Segurança

Os fatores de segurança mínimos estabelecidos para os estudos de estabilidade são os fornecidos na tabela 5.2 a seguir

**Tabela 5.2 - Fatores de Segurança Admitidos**

CONDIÇÕES DE CARREGAMENTO	TALUDE	FATOR DE SEGURANÇA
Final de Construção	Montante e Jusante	1,3
Regime Permanente	Jusante	1,5
Rebaixamento Rápido	Montante	1,1

Estas condições haviam sido definidas no relatório RHAR-970515-RE - Critérios de Projeto

### 5.3 - Parâmetros Geotécnicos

Os parâmetros de resistência ao cisalhamento (ângulo de atrito e coesão) foram obtidos de ensaios triaxiais adensados, não drenados, saturados, com medida de poropressão ( $CIU_{sat}$ ) e ensaios de cisalhamento direto lento, executados nos materiais de empréstimo e materiais da fundação, cujos resultados foram apresentados no relatório RHAR-970930-RE - rev 1 - Estudos Geológico - Geotécnicos

Para os materiais integrantes da seção analisada, dispõe-se nesta fase de resultados de ensaios no material do maciço compactado e solo residual da fundação, subjacente ao aluvião. No primeiro caso, os parâmetros foram obtidos de ensaios de cisalhamento direto lento, enquanto no segundo, a partir de ensaios triaxiais  $CIU_{sat}$  realizados em corpos de prova talhados de amostras indeformadas tipo bloco.

Os parâmetros dos demais materiais, constituídos pelo filtro/tapete de areia e aluvião, foram estimados. No caso da areia do sistema de drenagem, a partir de experiência em obras anteriores e, no caso dos aluviões, a partir da classificação de campo e ensaios SPT.

Para análise do talude de montante, foi necessário ter em conta a contribuição física da ensecadeira e do material de enchimento a ser aplicado entre esta e o maciço da barragem. Como o controle construtivo nesses materiais de aterro será menos rigoroso que no maciço da barragem, foram considerados parâmetros minorados nestes casos.

A tabela 5.3 abaixo resume os valores adotados em cada caso.

**Tabela 5.3 - Parâmetros Geotécnicos Adotados**

MATERIAL	$\gamma$ ( $tf/m^3$ )	$c'$ ( $tf/m^2$ )	$\phi'$ ( $^\circ$ )	$r_u$	OBSERVAÇÃO
Aterro (Barragem)	2,0	2,0 *	31	0,20	Cis Direto Lento
Aterro (Ensecadeira)	2,0	1,0	28	0,20	Estimado
Aterro (Enchimento)	1,9	0,0	20	0,20	Estimado
Filtro (Areia)	1,8	0,0	30	-	Estimado
Aluvião	1,8	1,5	28	-	Estimado
Solo Residual	1,9	1,4	30	-	Triaxial $CIU_{sat}$

\* minorado nesta fase (valor obtido de ensaio =  $5,3 \text{ tf/m}^2$ )

### 5.4 - Resultados Obtidos

Os resultados obtidos nas análises estão apresentados nas listagens de computador apresentadas no final deste item. A tabela 4.4 a seguir relaciona os valores dos fatores de segurança referentes às superfícies críticas de escorregamento, ilustradas nas figuras apresentadas ao final de cada listagem de computador.

**Tabela 5.4 - Fatores de Segurança Obtidos**

CONDIÇÕES DE CARREGAMENTO	TALUDE	FATOR DE SEGURANÇA
Final de Construção	Montante	1,54
	Jusante	1,35
Regime Permanente	Jusante	1,51
Rebaixamento Rápido	Montante	1,13

Estes resultados demonstram que o talude de jusante está otimizado dentro dos critérios preconizados em ambas as situações de carregamento a que será submetido. No caso do talude de montante, sua inclinação é condicionada pelo rebaixamento rápido do nível d'água do reservatório.

DATA FILE NAME..... C:\GSLOPE\ARACO-03.GSL

Job No. Relatorio de Concepcao  
Title Barragem Aracoiaba  
Date 17/11/1997  
Label A Estaca 45 + 0,00  
Label B Talude Mon. - Final Construcao

Max Slice Width 10  
No. of Materials 7  
Seismic Acceleration .05  
External Forces 0  
Piezometric Surfaces 1  
Unit Wt. of Pore Fluid 1  
No. of Geogrid Layers 0  
FoS for Geogrid Pullout 0

Material	Unit Wt	Cohesion	Friction Angle	Piezo Surf.	Ru	Interaction Coefficient
1 Enchimento	1.9	0	20	1	.2	0
2 Ensecadeira	2	1	28	1	.2	0
3 Aterro Compact.	2	2	31	1	.2	0
4 Filtro de Areia	1.8	0	30	1	0	0
5 Aluviao	1.8	1.5	28	1	0	0
6 Solo Residual	1.9	1.4	30	1	0	0
7 Rocha	2.2	100	50	1	0	0

Upper Surface of Material # 1 (Enchimento)

X-Coord	Y-Coord
4	69
20	69
29	75
34	75
63	75
113	95
123	100
130	100
150	90

Upper Surface of Material # 2 (Ensecadeira)

X-Coord	Y-Coord
4	69
20	69
29	75
34	75
43	69
48	69
63	75
113	95
123	100
130	100
150	90

000069

Upper Surface of Material # 3 (Aterro Compact.)

X-Coord	Y-Coord
4	69
20	69
43	69
48	69
63	75
113	95
123	100
130	100
150	90

Upper Surface of Material # 4 (Filtro de Areia)

X-Coord	Y-Coord
4	69
20	69
43	69
48	69
49	67.5
95.5	67.5
99.5	63.5
100.5	62.5
125.5	62.5
126.5	63.5
130.5	67.5
131	95
132	95
132.5	69
150	69

Upper Surface of Material # 5 (Aluviao)

X-Coord	Y-Coord
4	69
20	69
43	69
48	69
49	67.5
95.5	67.5
99.5	63.5
100.5	62.5
125.5	62.5
126.5	63.5
130.5	67.5
150	67.5

Upper Surface of Material # 6 (Solo Residual)

X-Coord	Y-Coord
4	63.5
99.5	63.5
100.5	62.5
125.5	62.5
126.5	63.5
150	63.5

Upper Surface of Material # 7 (Rocha)

X-Coord	Y-Coord
4	62.5
54	62.5
125.5	62.5

630070

150                      62.5

Piezometric Surface No. 1	
X-Coord	Y-Coord
4	74
27.5	74
43	69
150	69

There are no explicit external forces in the data set.

Geogrid Layer No.	Horizontal Extents X1 <-----> X2	Geogrid Layer Elevation	Strength per unit width
-------------------------	--	-------------------------------	-------------------------------

GSLOPE 3.32

LIMIT EQUILIBRIUM SLOPE STABILITY ANALYSIS

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COBA - Lisboa

Results are for Bishop's Modified Method unless otherwise noted.

File C:\GSLOPE\ARACO-03.GSL Output dated 12-01-1997 at 15:55:51

Material	Unit Wt	Cohesion	Friction Angle	Piezo Surf.	Ru	Interaction Coefficient
1 Enchimento	1.9	0	20	1	.2	0
2 Ensecadeira	2	1	28	1	.2	0
3 Aterro Compact.	2	2	31	1	.2	0
4 Filtro de Areia	1.8	0	30	1	0	0
5 Aluviao	1.8	1.5	28	1	0	0
6 Solo Residual	1.9	1.4	30	1	0	0
7 Rocha	2.2	100	50	1	0	0

X-centre	Y-centre	Radius	Factor of Safety	Iterations	Slices	M Alpha Warnings
50.00	110.00	50.00	13.9446	3	15	0
50.00	110.00	52.00	15.1038	3	16	0
50.00	110.00	48.00	9.2127	4	15	0
50.00	110.00	46.00	3.2721	4	12	0
50.00	110.00	44.00	3.8612	4	12	0
		MIN THIS CENTRE		3.272		
52.00	110.00	50.00	13.2593	3	16	0
52.00	110.00	52.00	14.5762	3	18	0
52.00	110.00	48.00	8.8109	4	17	0
52.00	110.00	46.00	3.1054	4	12	0
52.00	110.00	44.00	3.5059	4	12	0
		MIN THIS CENTRE		3.105		
54.00	110.00	50.00	12.5877	3	17	0
54.00	110.00	52.00	13.9374	3	19	0
54.00	110.00	48.00	8.1489	4	17	0
54.00	110.00	46.00	2.9471	4	12	0
54.00	110.00	44.00	3.2118	4	12	0
		MIN THIS CENTRE		2.947		
50.00	112.00	52.00	13.4505	3	16	0
50.00	112.00	54.00	14.5770	3	16	0
50.00	112.00	50.00	8.9939	4	16	0
50.00	112.00	48.00	3.1233	4	12	0
50.00	112.00	46.00	3.6541	4	12	0
		MIN THIS CENTRE		3.123		
52.00	112.00	52.00	12.7999	3	16	0



52.00	112.00	54.00	14.0449	3	19	0
52.00	112.00	50.00	8.4478	4	18	0
52.00	112.00	48.00	2.9771	4	12	0
52.00	112.00	46.00	3.3502	4	12	0
		MIN THIS CENTRE	2.977			

54.00	112.00	52.00	12.1588	3	18	0
54.00	112.00	54.00	13.4431	3	19	0
54.00	112.00	50.00	7.8743	4	17	0
54.00	112.00	48.00	2.8365	4	13	0
54.00	112.00	46.00	3.0912	4	12	0
		MIN THIS CENTRE	2.836			

50.00	114.00	54.00	13.0310	3	16	0
50.00	114.00	56.00	14.1054	3	18	0
50.00	114.00	52.00	8.6332	4	17	0
50.00	114.00	50.00	2.9914	4	12	0
50.00	114.00	48.00	3.4743	4	12	0
		MIN THIS CENTRE	2.991			

52.00	114.00	54.00	12.3726	3	17	0
52.00	114.00	56.00	13.5561	3	19	0
52.00	114.00	52.00	8.1449	4	18	0
52.00	114.00	50.00	2.8634	4	12	0
52.00	114.00	48.00	3.2113	4	12	0
		MIN THIS CENTRE	2.863			

54.00	114.00	54.00	11.7520	3	18	0
54.00	114.00	56.00	12.9769	3	19	0
54.00	114.00	52.00	7.6098	4	18	0
54.00	114.00	50.00	2.7526	4	13	0
54.00	114.00	48.00	2.9809	4	12	0
		MIN THIS CENTRE	2.752			

SF!( 9 ) = 2.752553

56.00	112.00	48.00	2.7231	4	14	0
56.00	112.00	50.00	7.4289	4	19	0
56.00	112.00	46.00	2.8566	4	12	0
		MIN THIS CENTRE	2.723			

56.00	114.00	50.00	2.6456	4	15	0
56.00	114.00	52.00	7.1956	4	19	0
56.00	114.00	48.00	2.7910	4	13	0
		MIN THIS CENTRE	2.645			

52.00	116.00	52.00	2.7669	4	13	0
52.00	116.00	54.00	7.7675	4	17	0
52.00	116.00	50.00	3.0870	4	12	0
		MIN THIS CENTRE	2.766			

54.00	116.00	52.00	2.6718	4	13	0
54.00	116.00	54.00	7.3700	4	19	0
54.00	116.00	50.00	2.8804	4	12	0
		MIN THIS CENTRE	2.671			

56.00	116.00	52.00	2.5749	4	16	0
56.00	116.00	54.00	6.9705	4	19	0
56.00	116.00	50.00	2.7237	4	13	0
		MIN THIS CENTRE	2.574			

SF!( 9 ) = 2.574889

58.00	114.00	50.00	2.5133	4	15	0
58.00	114.00	52.00	6.7652	4	19	0
58.00	114.00	48.00	2.6122	4	12	0
		MIN THIS CENTRE	2.513			

58.00	116.00	52.00	2.4563	4	15	0
58.00	116.00	54.00	6.5504	4	19	0
58.00	116.00	50.00	2.5622	4	14	0
		MIN THIS CENTRE	2.456			

54.00	118.00	54.00	2.5961	4	15	0
54.00	118.00	56.00	7.1413	4	19	0
54.00	118.00	52.00	2.8057	4	13	0
		MIN THIS CENTRE	2.596			

56.00	118.00	54.00	2.5063	4	16	0
56.00	118.00	56.00	6.7532	4	19	0
56.00	118.00	52.00	2.6565	4	14	0
		MIN THIS CENTRE	2.506			

58.00	118.00	54.00	2.3998	4	15	0
58.00	118.00	56.00	6.3453	4	19	0
58.00	118.00	52.00	2.5140	4	15	0
		MIN THIS CENTRE	2.399			

SF!( 9 ) = 2.399836

60.00	116.00	52.00	2.3548	4	15	0
60.00	116.00	54.00	6.1477	4	19	0
60.00	116.00	50.00	2.4144	4	14	0
		MIN THIS CENTRE	2.354			

60.00	118.00	54.00	2.3023	4	15	0
60.00	118.00	56.00	6.0353	4	20	0
60.00	118.00	52.00	2.3705	4	14	0
		MIN THIS CENTRE	2.302			

56.00	120.00	56.00	2.4417	4	16	0
56.00	120.00	58.00	6.5419	4	19	0
56.00	120.00	54.00	2.5957	4	15	0
		MIN THIS CENTRE	2.441			

58.00	120.00	56.00	2.3444	4	15	0
58.00	120.00	58.00	6.1503	4	19	0
58.00	120.00	54.00	2.4639	4	15	0
		MIN THIS CENTRE	2.344			

60.00	120.00	56.00	2.2513	4	15	0
60.00	120.00	58.00	5.8731	4	20	0

60.00	120.00	54.00	2.3278	4	14	0
		MIN THIS CENTRE		2.251		

SF!( 9 ) = 2.251319

62.00	118.00	54.00	2.1973	4	14	0
62.00	118.00	56.00	5.6921	4	20	0
62.00	118.00	52.00	2.1797	4	18	0
62.00	118.00	50.00	2.0993	4	13	0
62.00	118.00	48.00	2.0780	4	13	0
62.00	118.00	46.00	2.0523	4	10	0
62.00	118.00	44.00	2.1604	5	7	0
		MIN THIS CENTRE		2.052		

62.00	120.00	56.00	2.1852	4	16	0
62.00	120.00	58.00	5.5727	4	20	0
62.00	120.00	54.00	2.1551	4	18	0
62.00	120.00	52.00	2.0673	4	13	0
62.00	120.00	50.00	2.0498	4	13	0
62.00	120.00	48.00	2.0246	4	10	0
62.00	120.00	46.00	2.1164	5	7	0
		MIN THIS CENTRE		2.024		

58.00	122.00	58.00	2.2904	4	15	0
58.00	122.00	60.00	6.0438	4	20	0
58.00	122.00	56.00	2.4137	4	15	0
		MIN THIS CENTRE		2.290		

60.00	122.00	58.00	2.2256	4	16	0
60.00	122.00	60.00	5.7296	4	21	0
60.00	122.00	56.00	2.2845	4	14	0
		MIN THIS CENTRE		2.225		

62.00	122.00	58.00	2.1503	4	17	0
62.00	122.00	60.00	5.4480	4	20	0
62.00	122.00	56.00	2.1283	4	18	0
62.00	122.00	54.00	2.0353	4	13	0
62.00	122.00	52.00	2.0213	4	13	0
62.00	122.00	50.00	1.9985	4	11	0
62.00	122.00	48.00	2.0818	5	8	0
		MIN THIS CENTRE		1.998		

SF!( 9 ) = 1.998502

64.00	120.00	48.00	1.9233	4	12	0
64.00	120.00	50.00	1.9540	4	13	0
64.00	120.00	46.00	1.9800	5	7	0
		MIN THIS CENTRE		1.923		

64.00	122.00	50.00	1.9011	4	12	0
64.00	122.00	52.00	1.9256	4	13	0
64.00	122.00	48.00	1.9545	4	7	0
		MIN THIS CENTRE		1.901		

60.00	124.00	52.00	2.0988	4	11	0
60.00	124.00	54.00	2.1057	4	13	0
60.00	124.00	50.00	2.2377	5	7	0
		MIN THIS CENTRE		2.098		

62.00	124.00	52.00	1.9743	4	12	0
62.00	124.00	54.00	1.9926	4	13	0
62.00	124.00	50.00	2.0539	5	8	0

MIN THIS CENTRE 1.974

64.00	124.00	52.00	1.8781	4	12	0
64.00	124.00	54.00	1.8972	4	13	0
64.00	124.00	50.00	1.9324	4	9	0

MIN THIS CENTRE 1.878

SF!( 9 ) = 1.878069

66.00	122.00	50.00	1.8209	4	11	0
66.00	122.00	52.00	1.8420	4	12	0
66.00	122.00	48.00	1.8636	4	9	0

MIN THIS CENTRE 1.820

66.00	124.00	52.00	1.7973	4	11	0
66.00	124.00	54.00	1.8390	4	13	0
66.00	124.00	50.00	1.8431	4	9	0

MIN THIS CENTRE 1.797

62.00	126.00	54.00	1.9499	4	12	0
62.00	126.00	56.00	1.9637	4	13	0
62.00	126.00	52.00	2.0242	5	8	0

MIN THIS CENTRE 1.949

64.00	126.00	54.00	1.8543	4	12	0
64.00	126.00	56.00	1.8690	4	13	0
64.00	126.00	52.00	1.9102	4	10	0

MIN THIS CENTRE 1.854

66.00	126.00	54.00	1.7733	4	11	0
66.00	126.00	56.00	1.8180	4	14	0
66.00	126.00	52.00	1.8214	4	9	0

MIN THIS CENTRE 1.773

SF!( 9 ) = 1.773333

68.00	124.00	52.00	1.7259	4	10	0
68.00	124.00	54.00	1.7813	4	14	0
68.00	124.00	50.00	1.7713	4	9	0

MIN THIS CENTRE 1.725

68.00	126.00	54.00	1.7311	4	11	0
68.00	126.00	56.00	1.7662	4	14	0
68.00	126.00	52.00	1.7481	4	9	0

MIN THIS CENTRE 1.731

64.00	128.00	56.00	1.8301	4	12	0
64.00	128.00	58.00	1.8643	4	14	0
64.00	128.00	54.00	1.8873	4	10	0

MIN THIS CENTRE 1.830

66.00	128.00	56.00	1.7493	4	12	0
66.00	128.00	58.00	1.8012	4	15	0
66.00	128.00	54.00	1.7987	4	9	0
		MIN THIS CENTRE	1.749			

68.00	128.00	56.00	1.7156	4	12	0
68.00	128.00	58.00	1.7491	4	14	0
68.00	128.00	54.00	1.7245	4	9	0
		MIN THIS CENTRE	1.715			

SF!( 9 ) = 1.715594

70.00	126.00	54.00	1.6919	4	12	0
70.00	126.00	56.00	1.7195	4	14	0
70.00	126.00	52.00	1.7142	4	10	0
		MIN THIS CENTRE	1.691			

70.00	128.00	56.00	1.6779	4	12	0
70.00	128.00	58.00	1.6991	4	14	0
70.00	128.00	54.00	1.6967	4	10	0
		MIN THIS CENTRE	1.677			

66.00	130.00	58.00	1.7520	4	13	0
66.00	130.00	60.00	1.7848	4	15	0
66.00	130.00	56.00	1.7754	4	9	0
		MIN THIS CENTRE	1.752			

68.00	130.00	58.00	1.7037	4	12	0
68.00	130.00	60.00	1.7303	4	14	0
68.00	130.00	56.00	1.7264	4	10	0
		MIN THIS CENTRE	1.703			

70.00	130.00	58.00	1.6619	4	12	0
70.00	130.00	60.00	1.6775	4	14	0
70.00	130.00	56.00	1.6846	4	11	0
		MIN THIS CENTRE	1.661			

SF!( 9 ) = 1.661876

72.00	128.00	56.00	1.6412	4	12	0
72.00	128.00	58.00	1.6515	4	13	0
72.00	128.00	54.00	1.6700	4	11	0
		MIN THIS CENTRE	1.641			

72.00	130.00	58.00	1.6211	4	12	0
72.00	130.00	60.00	1.6476	4	15	0
72.00	130.00	56.00	1.6542	4	11	0
		MIN THIS CENTRE	1.621			

68.00	132.00	60.00	1.6898	4	12	0
68.00	132.00	62.00	1.7100	4	14	0
68.00	132.00	58.00	1.7092	4	10	0
		MIN THIS CENTRE	1.689			

70.00	132.00	60.00	1.6440	4	12	0
70.00	132.00	62.00	1.6685	4	16	0

70.00	132.00	58.00	1.6720	4	11	0
		MIN THIS CENTRE		1.644		
72.00	132.00	60.00	1.6000	4	12	0
72.00	132.00	62.00	1.6336	4	16	0
72.00	132.00	58.00	1.6363	4	11	0
		MIN THIS CENTRE		1.599		
SF!( 9 ) = 1.599998						
74.00	130.00	58.00	1.5990	4	13	0
74.00	130.00	60.00	1.6232	4	15	0
74.00	130.00	56.00	1.6215	4	9	0
		MIN THIS CENTRE		1.599		
74.00	132.00	60.00	1.5849	4	13	0
74.00	132.00	62.00	1.6147	4	16	0
74.00	132.00	58.00	1.5992	4	9	0
		MIN THIS CENTRE		1.584		
70.00	134.00	62.00	1.6247	4	12	0
70.00	134.00	64.00	1.6519	4	16	0
70.00	134.00	60.00	1.6570	4	11	0
		MIN THIS CENTRE		1.624		
72.00	134.00	62.00	1.5959	4	14	0
72.00	134.00	64.00	1.6230	4	17	0
72.00	134.00	60.00	1.6165	4	11	0
		MIN THIS CENTRE		1.595		
74.00	134.00	62.00	1.5741	4	14	0
74.00	134.00	64.00	1.6087	4	16	0
74.00	134.00	60.00	1.5927	4	11	0
		MIN THIS CENTRE		1.574		
SF!( 9 ) = 1.574105						
76.00	132.00	60.00	1.5738	4	15	0
76.00	132.00	62.00	1.6082	4	16	0
76.00	132.00	58.00	1.5862	4	11	0
		MIN THIS CENTRE		1.573		
76.00	134.00	62.00	1.5677	4	15	0
76.00	134.00	64.00	1.6047	4	17	0
76.00	134.00	60.00	1.5731	4	11	0
		MIN THIS CENTRE		1.567		
72.00	136.00	64.00	1.5825	4	14	0
72.00	136.00	66.00	1.6152	4	18	0
72.00	136.00	62.00	1.5955	4	11	0
		MIN THIS CENTRE		1.582		
74.00	136.00	64.00	1.5665	4	15	0
74.00	136.00	66.00	1.6045	4	17	0
74.00	136.00	62.00	1.5767	4	13	0
		MIN THIS CENTRE		1.566		

76.00	136.00	64.00	1.5637	4	15	0
76.00	136.00	66.00	1.6034	4	18	0
76.00	136.00	62.00	1.5638	4	12	0
		MIN THIS CENTRE	1.563			

SF!( 9 ) = 1.563717

78.00	134.00	62.00	1.5731	4	15	0
78.00	134.00	64.00	1.6126	4	19	0
78.00	134.00	60.00	1.5697	4	13	0
78.00	134.00	58.00	1.6059	4	10	0
		MIN THIS CENTRE	1.569			

78.00	136.00	64.00	1.5713	4	17	0
78.00	136.00	66.00	1.6156	4	20	0
78.00	136.00	62.00	1.5649	4	13	0
78.00	136.00	60.00	1.5939	4	11	0
		MIN THIS CENTRE	1.564			

74.00	138.00	66.00	1.5613	4	15	0
74.00	138.00	68.00	1.6016	4	18	0
74.00	138.00	64.00	1.5644	4	13	0
		MIN THIS CENTRE	1.561			

76.00	138.00	66.00	1.5614	4	15	0
76.00	138.00	68.00	1.6049	4	19	0
76.00	138.00	64.00	1.5574	4	13	0
76.00	138.00	62.00	1.6030	4	10	0
		MIN THIS CENTRE	1.557			

78.00	138.00	66.00	1.5724	4	18	0
78.00	138.00	68.00	1.6208	4	21	0
78.00	138.00	64.00	1.5620	4	13	0
78.00	138.00	62.00	1.5856	4	12	0
		MIN THIS CENTRE	1.562			

SF!( 8 ) = 1.557416

74.00	140.00	66.00	1.5556	4	14	0
74.00	140.00	68.00	1.5580	4	15	0
74.00	140.00	64.00	1.6227	4	10	0
		MIN THIS CENTRE	1.555			

76.00	140.00	66.00	1.5532	4	13	0
76.00	140.00	68.00	1.5605	4	17	0
76.00	140.00	64.00	1.5910	4	11	0
		MIN THIS CENTRE	1.553			

78.00	140.00	66.00	1.5605	4	14	0
78.00	140.00	68.00	1.5758	4	19	0
78.00	140.00	64.00	1.5801	4	12	0
		MIN THIS CENTRE	1.560			

SF!( 8 ) = 1.553224

74.00	142.00	68.00	1.5494	4	15	0
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74.00	142.00	70.00	1.5562	4	16	0
74.00	142.00	66.00	1.6060	4	10	0
		MIN THIS CENTRE	1.549			

76.00	142.00	68.00	1.5508	4	13	0
76.00	142.00	70.00	1.5621	4	18	0
76.00	142.00	66.00	1.5826	4	12	0
		MIN THIS CENTRE	1.550			

78.00	142.00	68.00	1.5611	4	16	0
78.00	142.00	70.00	1.5813	4	20	0
78.00	142.00	66.00	1.5768	4	12	0
		MIN THIS CENTRE	1.561			

SF!( 7 ) = 1.549431

72.00	140.00	66.00	1.5731	4	13	0
72.00	140.00	68.00	1.5653	4	16	0
72.00	140.00	70.00	1.6058	4	18	0
		MIN THIS CENTRE	1.565			

72.00	142.00	68.00	1.5611	4	13	0
72.00	142.00	70.00	1.5603	4	16	0
72.00	142.00	72.00	1.6032	4	19	0
		MIN THIS CENTRE	1.560			

72.00	144.00	70.00	1.5524	4	14	0
72.00	144.00	72.00	1.5571	4	16	0
72.00	144.00	68.00	1.6329	4	10	0
		MIN THIS CENTRE	1.552			

74.00	144.00	70.00	1.5454	4	15	0
74.00	144.00	72.00	1.5554	4	18	0
74.00	144.00	68.00	1.5935	4	11	0
		MIN THIS CENTRE	1.545			

76.00	144.00	70.00	1.5496	4	14	0
76.00	144.00	72.00	1.5659	4	19	0
76.00	144.00	68.00	1.5769	4	12	0
		MIN THIS CENTRE	1.549			

SF!( 8 ) = 1.545435

72.00	146.00	72.00	1.5464	4	15	0
72.00	146.00	74.00	1.5554	4	16	0
72.00	146.00	70.00	1.6150	4	10	0
		MIN THIS CENTRE	1.546			

74.00	146.00	72.00	1.5432	4	15	0
74.00	146.00	74.00	1.5571	4	19	0
74.00	146.00	70.00	1.5844	4	12	0
		MIN THIS CENTRE	1.543			

76.00	146.00	72.00	1.5502	4	15	0
76.00	146.00	74.00	1.5715	4	20	0
76.00	146.00	70.00	1.5734	4	12	0



MIN THIS CENTRE 1.550

SF!( 8 ) = 1.543162

72.00	148.00	74.00	1.5423	4	15	0
72.00	148.00	76.00	1.5547	4	18	0
72.00	148.00	72.00	1.6012	4	11	0
		MIN THIS CENTRE	1.542			

74.00	148.00	74.00	1.5420	4	16	0
74.00	148.00	76.00	1.5607	4	20	0
74.00	148.00	72.00	1.5781	4	12	0
		MIN THIS CENTRE	1.541			

76.00	148.00	74.00	1.5531	4	16	0
76.00	148.00	76.00	1.5786	4	20	0
76.00	148.00	72.00	1.5716	4	12	0
		MIN THIS CENTRE	1.553			

SF!( 8 ) = 1.541995

72.00	150.00	76.00	1.5400	4	15	0
72.00	150.00	78.00	1.5561	4	19	0
72.00	150.00	74.00	1.5910	4	12	0
		MIN THIS CENTRE	1.540			

74.00	150.00	76.00	1.5423	4	17	0
74.00	150.00	78.00	1.5660	4	21	0
74.00	150.00	74.00	1.5740	4	12	0
		MIN THIS CENTRE	1.542			

76.00	150.00	76.00	1.5578	4	17	0
76.00	150.00	78.00	1.5863	4	22	0
76.00	150.00	74.00	1.5707	4	13	0
		MIN THIS CENTRE	1.557			

SF!( 7 ) = 1.540002

70.00	148.00	74.00	1.5554	4	14	0
70.00	148.00	76.00	1.5614	4	17	0
70.00	148.00	72.00	1.6502	4	10	0
		MIN THIS CENTRE	1.555			

70.00	150.00	76.00	1.5489	4	15	0
70.00	150.00	78.00	1.5594	4	17	0
70.00	150.00	74.00	1.6302	4	10	0
		MIN THIS CENTRE	1.548			

70.00	152.00	78.00	1.5444	4	15	0
70.00	152.00	80.00	1.5583	4	19	0
70.00	152.00	76.00	1.6145	4	10	0
		MIN THIS CENTRE	1.544			

72.00	152.00	78.00	1.5389	4	16	0
72.00	152.00	80.00	1.5594	4	19	0
72.00	152.00	76.00	1.5837	4	12	0
		MIN THIS CENTRE	1.538			

74.00	152.00	78.00	1.5448	4	18	0
74.00	152.00	80.00	1.5728	4	21	0
74.00	152.00	76.00	1.5717	4	12	0
		MIN THIS CENTRE	1.544			

SF!( 8 ) = 1.538873

70.00	154.00	80.00	1.5416	4	15	0
70.00	154.00	82.00	1.5591	4	20	0
70.00	154.00	78.00	1.6027	4	11	0
		MIN THIS CENTRE	1.541			

72.00	154.00	80.00	1.5387	4	17	0
72.00	154.00	82.00	1.5643	4	21	0
72.00	154.00	78.00	1.5786	4	12	0
		MIN THIS CENTRE	1.538			

74.00	154.00	80.00	1.5490	4	19	0
74.00	154.00	82.00	1.5807	4	21	0
74.00	154.00	78.00	1.5704	4	13	0
		MIN THIS CENTRE	1.549			

SF!( 8 ) = 1.538673

70.00	156.00	82.00	1.5402	4	15	0
70.00	156.00	84.00	1.5618	4	21	0
70.00	156.00	80.00	1.5938	4	12	0
		MIN THIS CENTRE	1.540			

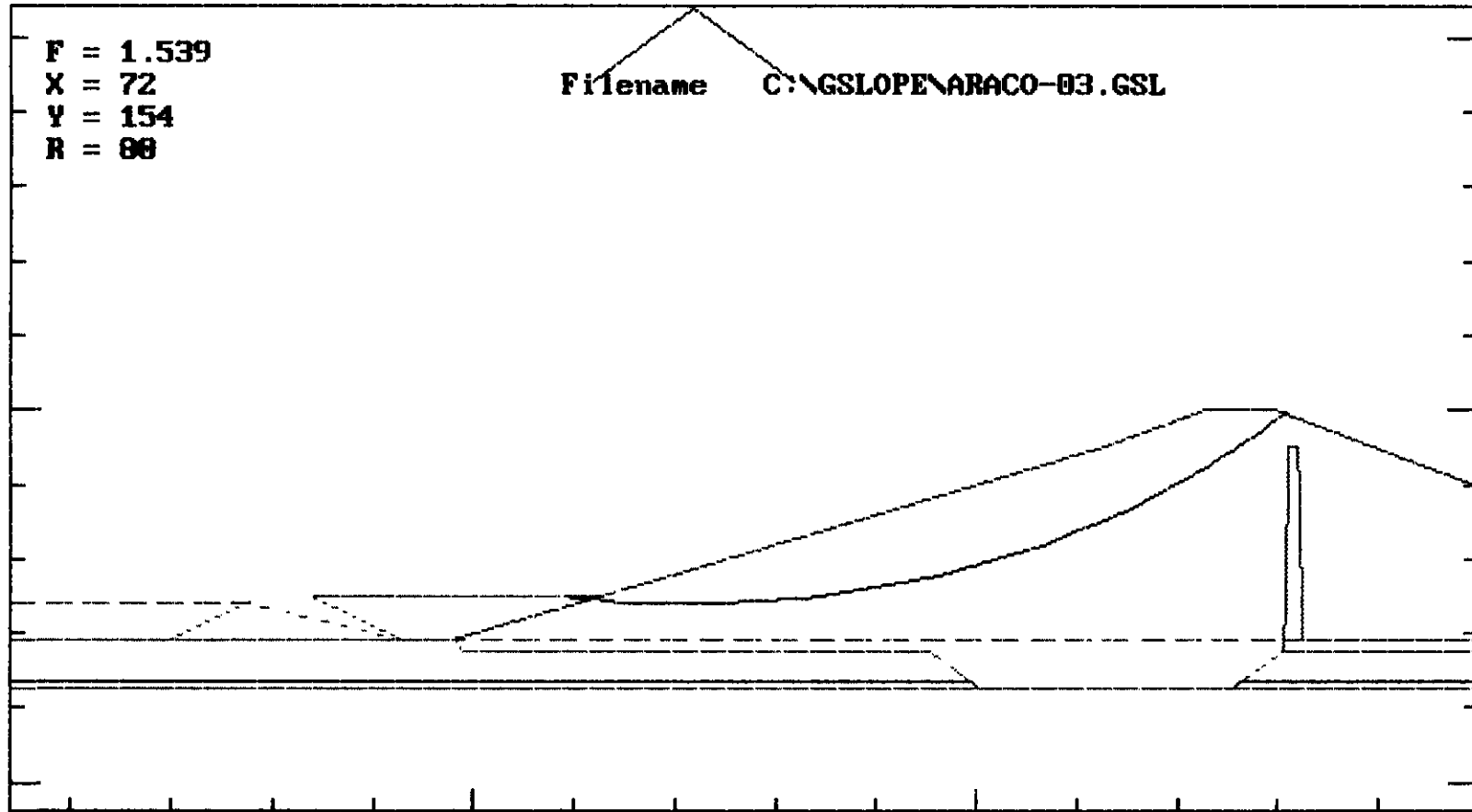
72.00	156.00	82.00	1.5405	4	18	0
72.00	156.00	84.00	1.5706	4	21	0
72.00	156.00	80.00	1.5754	4	12	0
		MIN THIS CENTRE	1.540			

74.00	156.00	82.00	1.5548	4	20	0
74.00	156.00	84.00	1.5881	4	23	0
74.00	156.00	80.00	1.5703	4	14	0
		MIN THIS CENTRE	1.554			

SF!( 5 ) = 1.538673

Minimum Bishop Factor of Safety this run:

72.00	154.00	80.00	1.5387	4	17	0
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DATA FILE NAME..... C:\GSLOPE\ARACO-01.GSL

Job No. Relatorio de Concepcao  
Title Barragem Aracoiaba  
Date 14/11/1997  
Label A Estaca 45 + 0,00  
Label B Talude Jus. - Final Construcao

Max Slice Width 10  
No. of Materials 6  
Seismic Acceleration .05  
External Forces 0  
Piezometric Surfaces 1  
Unit Wt. of Pore Fluid 1  
No. of Geogrid Layers 0  
FoS for Geogrid Pullout 0

Material	Unit Wt	Cohesion	Friction Angle	Piezo Surf.	Ru	Interaction Coefficient
1 Enrocamento	2.1	0	38	1	0	0
2 Aterro Compact.	2	2	31	1	.2	0
3 Filtro de Areia	1.8	0	30	1	0	0
4 Aluviao	1.8	1.5	28	1	0	0
5 Solo Residual	1.9	1.4	30	1	0	0
6 Rocha	2.2	100	50	1	0	0

Upper Surface of Material # 1 (Enrocamento)

X-Coord	Y-Coord
0	91.4
9	95
19	100
26	100
46	90
49	90
69	80
72	80
92	70
97	70
101	68
115	68
130	68

Upper Surface of Material # 2 (Aterro Compact.)

X-Coord	Y-Coord
0	91.4
9	95
19	100
26	100
46	90
49	90
69	80
72	80
92	70

94	69
97	67.5
105.5	63.5
110.5	63.5
115	68
130	68

Upper Surface of Material # 3 (Filtro de Areia)

X-Coord	Y-Coord
0	63.5
22.5	63.5
26.5	67.5
27	95
28	95
28.5	69
94	69
97	67.5
105.5	63.5
110.5	63.5
115	68
130	68

Upper Surface of Material # 4 (Aluviao)

X-Coord	Y-Coord
0	63.5
22.5	63.5
26.5	67.5
97	67.5
105.5	63.5
110.5	63.5
130	63.5

Upper Surface of Material # 5 (Solo Residual)

X-Coord	Y-Coord
0	63.5
22.5	63.5
105.5	63.5
110.5	63.5
130	63.5

Upper Surface of Material # 6 (Rocha)

X-Coord	Y-Coord
0	62.5
22	62.5
130	62.5

Piezometric Surface No. 1

X-Coord	Y-Coord
0	68
130	68

There are no explicit external forces in the data set.

Geogrid Layer No.	Horizontal Extents X1 <-----> X2	Geogrid Layer Elevation	Strength per unit width
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GSLOPE 3.32

LIMIT EQUILIBRIUM SLOPE STABILITY ANALYSIS

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COBA - Lisboa

Results are for Bishop's Modified Method unless otherwise noted.

File C:\GSLOPE\ARACO-01.GSL Output dated 12-02-1997 at 09:42:47

Material	Unit Wt	Cohesion	Friction Angle	Piezo Surf.	Ru	Interaction Coefficient
1 Enrocamento	2.1	0	38	1	0	0
2 Aterro Compact.	2	2	31	1	.2	0
3 Filtro de Areia	1.8	0	30	1	0	0
4 Aluviao	1.8	1.5	28	1	0	0
5 Solo Residual	1.9	1.4	30	1	0	0
6 Rocha	2.2	100	50	1	0	0

X-centre	Y-centre	Radius	Factor of Safety	Iterations	Slices	M Alpha Warnings
50.00	110.00	50.00	7.5411	3	24	0
50.00	110.00	52.00	8.9726	3	25	0
50.00	110.00	48.00	4.8560	4	23	0
50.00	110.00	46.00	2.4092	4	23	0
50.00	110.00	44.00	2.3334	4	22	0
50.00	110.00	42.00	2.2390	4	19	0
50.00	110.00	40.00	2.0874	4	18	0
50.00	110.00	38.00	2.0059	4	18	0
50.00	110.00	36.00	1.9215	4	17	0
50.00	110.00	34.00	1.8853	4	15	0
50.00	110.00	32.00	1.8297	4	14	0
50.00	110.00	30.00	1.7836	4	14	0
50.00	110.00	28.00	1.7587	4	12	0
50.00	110.00	26.00	1.8471	4	8	0
		MIN THIS CENTRE		1.758		
52.00	110.00	50.00	7.2247	3	23	0
52.00	110.00	52.00	8.5896	3	25	0
52.00	110.00	48.00	4.6395	4	24	0
52.00	110.00	46.00	2.2663	4	24	0
52.00	110.00	44.00	2.1962	4	20	0
52.00	110.00	42.00	2.1291	4	19	0
52.00	110.00	40.00	1.9944	4	18	0
52.00	110.00	38.00	1.9201	4	18	0
52.00	110.00	36.00	1.8485	4	17	0
52.00	110.00	34.00	1.8156	4	14	0
52.00	110.00	32.00	1.7693	4	14	0
52.00	110.00	30.00	1.7383	4	12	0
52.00	110.00	28.00	1.7479	4	10	0
		MIN THIS CENTRE		1.738		
54.00	110.00	50.00	6.9327	3	25	0

54.00	110.00	52.00	8.2534	3	25	0
54.00	110.00	48.00	4.4420	4	27	0
54.00	110.00	46.00	2.1287	4	21	0
54.00	110.00	44.00	2.1024	4	20	0
54.00	110.00	42.00	2.0375	4	19	0
54.00	110.00	40.00	1.9152	4	18	0
54.00	110.00	38.00	1.8490	4	18	0
54.00	110.00	36.00	1.7903	4	17	0
54.00	110.00	34.00	1.7295	4	16	0
54.00	110.00	32.00	1.7242	4	12	0
54.00	110.00	30.00	1.7193	4	10	0
54.00	110.00	28.00	1.7368	4	6	0

MIN THIS CENTRE 1.719

50.00	112.00	52.00	7.5569	3	24	0
50.00	112.00	54.00	8.9861	3	25	0
50.00	112.00	50.00	4.8612	4	23	0
50.00	112.00	48.00	2.4117	4	23	0
50.00	112.00	46.00	2.3381	4	22	0
50.00	112.00	44.00	2.2321	4	19	0
50.00	112.00	42.00	2.0827	4	18	0
50.00	112.00	40.00	2.0014	4	18	0
50.00	112.00	38.00	1.9163	4	17	0
50.00	112.00	36.00	1.8733	4	15	0
50.00	112.00	34.00	1.8126	4	15	0
50.00	112.00	32.00	1.7679	4	14	0
50.00	112.00	30.00	1.7360	4	12	0
50.00	112.00	28.00	1.8045	4	11	0

MIN THIS CENTRE 1.735

52.00	112.00	52.00	7.2407	3	23	0
52.00	112.00	54.00	8.6024	3	25	0
52.00	112.00	50.00	4.6375	4	25	0
52.00	112.00	48.00	2.2647	4	24	0
52.00	112.00	46.00	2.1900	4	20	0
52.00	112.00	44.00	2.1217	4	19	0
52.00	112.00	42.00	1.9875	4	18	0
52.00	112.00	40.00	1.9125	4	18	0
52.00	112.00	38.00	1.8396	4	18	0
52.00	112.00	36.00	1.7971	4	15	0
52.00	112.00	34.00	1.7514	4	14	0
52.00	112.00	32.00	1.7154	4	13	0
52.00	112.00	30.00	1.7120	4	12	0
52.00	112.00	28.00	1.7753	5	8	0

MIN THIS CENTRE 1.712

54.00	112.00	52.00	6.9429	3	25	0
54.00	112.00	54.00	8.2644	3	25	0
54.00	112.00	50.00	4.4493	4	25	0
54.00	112.00	48.00	2.1187	4	21	0
54.00	112.00	46.00	2.0944	4	20	0
54.00	112.00	44.00	2.0285	4	19	0
54.00	112.00	42.00	1.9056	4	18	0
54.00	112.00	40.00	1.8375	4	18	0
54.00	112.00	38.00	1.7749	4	18	0
54.00	112.00	36.00	1.7152	4	16	0
54.00	112.00	34.00	1.7009	4	13	0
54.00	112.00	32.00	1.6845	4	12	0
54.00	112.00	30.00	1.7180	4	8	0

MIN THIS CENTRE 1.684

50.00	114.00	54.00	7.5814	3	24	0
50.00	114.00	56.00	9.0407	3	26	0
50.00	114.00	52.00	4.8723	4	23	0
50.00	114.00	50.00	2.4156	4	21	0
50.00	114.00	48.00	2.3458	4	22	0
50.00	114.00	46.00	2.2639	4	21	0
50.00	114.00	44.00	2.0833	4	18	0
50.00	114.00	42.00	2.0025	4	18	0
50.00	114.00	40.00	1.9183	4	17	0
50.00	114.00	38.00	1.8695	4	15	0
50.00	114.00	36.00	1.8059	4	15	0
50.00	114.00	34.00	1.7568	4	14	0
50.00	114.00	32.00	1.7268	4	14	0
50.00	114.00	30.00	1.7708	4	11	0
		MIN THIS CENTRE	1.726			

52.00	114.00	54.00	7.2637	3	24	0
52.00	114.00	56.00	8.6246	3	25	0
52.00	114.00	52.00	4.6484	4	25	0
52.00	114.00	50.00	2.2665	4	24	0
52.00	114.00	48.00	2.2187	4	22	0
52.00	114.00	46.00	2.1196	4	19	0
52.00	114.00	44.00	1.9862	4	18	0
52.00	114.00	42.00	1.9110	4	18	0
52.00	114.00	40.00	1.8373	4	18	0
52.00	114.00	38.00	1.7883	4	15	0
52.00	114.00	36.00	1.7382	4	15	0
52.00	114.00	34.00	1.7039	4	14	0
52.00	114.00	32.00	1.6906	4	12	0
52.00	114.00	30.00	1.7470	4	10	0
		MIN THIS CENTRE	1.690			

54.00	114.00	54.00	6.9612	3	24	0
54.00	114.00	56.00	8.2853	3	25	0
54.00	114.00	52.00	4.4596	4	25	0
54.00	114.00	50.00	2.1396	4	24	0
54.00	114.00	48.00	2.0913	4	20	0
54.00	114.00	46.00	2.0250	4	19	0
54.00	114.00	44.00	1.9018	4	18	0
54.00	114.00	42.00	1.8327	4	18	0
54.00	114.00	40.00	1.7678	4	18	0
54.00	114.00	38.00	1.7056	4	17	0
54.00	114.00	36.00	1.6875	4	14	0
54.00	114.00	34.00	1.6651	4	12	0
54.00	114.00	32.00	1.6906	4	10	0
		MIN THIS CENTRE	1.665			

SF!( 9 ) = 1.665084

56.00	112.00	32.00	1.6810	4	9	0
56.00	112.00	34.00	1.6677	4	12	0
56.00	112.00	36.00	1.6787	4	16	0
		MIN THIS CENTRE	1.667			

56.00	114.00	34.00	1.6530	4	12	0
56.00	114.00	36.00	1.6478	4	12	0
56.00	114.00	38.00	1.6673	4	17	0
		MIN THIS CENTRE	1.647			



52.00	116.00	36.00	1.6966	4	14	0
52.00	116.00	38.00	1.7326	4	15	0
52.00	116.00	34.00	1.6792	4	13	0
52.00	116.00	32.00	1.7195	4	12	0

MIN THIS CENTRE 1.679

54.00	116.00	36.00	1.6535	4	14	0
54.00	116.00	38.00	1.6782	4	14	0
54.00	116.00	34.00	1.6629	4	12	0

MIN THIS CENTRE 1.653

56.00	116.00	36.00	1.6287	4	12	0
56.00	116.00	38.00	1.6357	4	14	0
56.00	116.00	34.00	1.6838	4	9	0

MIN THIS CENTRE 1.628

SF!( 9 ) = 1.628724

58.00	114.00	34.00	1.6559	4	7	0
58.00	114.00	36.00	1.6043	4	14	0
58.00	114.00	38.00	1.6338	4	15	0

MIN THIS CENTRE 1.604

58.00	116.00	36.00	1.6345	4	10	0
58.00	116.00	38.00	1.5888	4	14	0
58.00	116.00	40.00	1.6237	4	17	0

MIN THIS CENTRE 1.588

54.00	118.00	38.00	1.6481	4	14	0
54.00	118.00	40.00	1.6728	4	15	0
54.00	118.00	36.00	1.6476	4	12	0
54.00	118.00	34.00	1.7292	4	10	0

MIN THIS CENTRE 1.647

56.00	118.00	38.00	1.6157	4	12	0
56.00	118.00	40.00	1.6287	4	14	0
56.00	118.00	36.00	1.6578	4	10	0

MIN THIS CENTRE 1.615

58.00	118.00	38.00	1.6108	4	12	0
58.00	118.00	40.00	1.5803	4	15	0
58.00	118.00	42.00	1.6173	4	17	0

MIN THIS CENTRE 1.580

SF!( 9 ) = 1.580251

60.00	116.00	38.00	1.6002	4	13	0
60.00	116.00	40.00	1.5993	4	15	0
60.00	116.00	42.00	1.6261	4	17	0

MIN THIS CENTRE 1.599

60.00	118.00	40.00	1.5791	4	15	0
60.00	118.00	42.00	1.5884	4	16	0
60.00	118.00	38.00	1.6269	4	8	0

MIN THIS CENTRE 1.579

56.00	120.00	42.00	1.6246	4	14	0
56.00	120.00	44.00	1.6563	4	18	0
56.00	120.00	40.00	1.6105	4	14	0
56.00	120.00	38.00	1.6350	4	12	0
		MIN THIS CENTRE	1.610			

58.00	120.00	42.00	1.5778	4	16	0
58.00	120.00	44.00	1.6135	4	18	0
58.00	120.00	40.00	1.5918	4	12	0
		MIN THIS CENTRE	1.577			

60.00	120.00	42.00	1.5668	4	15	0
60.00	120.00	44.00	1.5828	4	17	0
60.00	120.00	40.00	1.6053	4	10	0
		MIN THIS CENTRE	1.566			

SF!( 9 ) = 1.566847

62.00	118.00	40.00	1.5934	4	12	0
62.00	118.00	42.00	1.5731	4	15	0
62.00	118.00	44.00	1.5889	4	17	0
		MIN THIS CENTRE	1.573			

62.00	120.00	42.00	1.5745	4	13	0
62.00	120.00	44.00	1.5617	4	15	0
62.00	120.00	46.00	1.5818	4	17	0
		MIN THIS CENTRE	1.561			

58.00	122.00	44.00	1.5781	4	16	0
58.00	122.00	46.00	1.6135	4	18	0
58.00	122.00	42.00	1.5834	4	13	0
		MIN THIS CENTRE	1.578			

60.00	122.00	44.00	1.5608	4	17	0
60.00	122.00	46.00	1.5796	4	17	0
60.00	122.00	42.00	1.5856	4	12	0
		MIN THIS CENTRE	1.560			

62.00	122.00	44.00	1.5562	4	15	0
62.00	122.00	46.00	1.5552	4	17	0
62.00	122.00	48.00	1.5771	4	17	0
		MIN THIS CENTRE	1.555			

SF!( 9 ) = 1.555244

64.00	120.00	44.00	1.5597	4	15	0
64.00	120.00	46.00	1.5591	4	15	0
64.00	120.00	48.00	1.5839	4	17	0
		MIN THIS CENTRE	1.559			

64.00	122.00	46.00	1.5434	4	15	0
64.00	122.00	48.00	1.5521	4	17	0
64.00	122.00	44.00	1.5776	4	13	0
		MIN THIS CENTRE	1.543			

60.00	124.00	48.00	1.5785	4	18	0
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60.00	124.00	50.00	1.6139	4	18	0
60.00	124.00	46.00	1.5589	4	17	0
60.00	124.00	44.00	1.5686	4	12	0
		MIN THIS CENTRE	1.558			

62.00	124.00	48.00	1.5521	4	17	0
62.00	124.00	50.00	1.5748	4	18	0
62.00	124.00	46.00	1.5462	4	15	0
62.00	124.00	44.00	1.5905	4	10	0
		MIN THIS CENTRE	1.546			

64.00	124.00	48.00	1.5348	4	16	0
64.00	124.00	50.00	1.5478	4	17	0
64.00	124.00	46.00	1.5594	4	13	0
		MIN THIS CENTRE	1.534			

SF!( 9 ) = 1.534821

66.00	122.00	46.00	1.5539	4	13	0
66.00	122.00	48.00	1.5371	4	15	0
66.00	122.00	50.00	1.5405	4	18	0
		MIN THIS CENTRE	1.537			

66.00	124.00	48.00	1.5367	4	15	0
66.00	124.00	50.00	1.5286	4	16	0
66.00	124.00	52.00	1.5357	4	18	0
		MIN THIS CENTRE	1.528			

62.00	126.00	50.00	1.5510	4	17	0
62.00	126.00	52.00	1.5763	4	18	0
62.00	126.00	48.00	1.5422	4	17	0
62.00	126.00	46.00	1.5736	4	10	0
		MIN THIS CENTRE	1.542			

64.00	126.00	50.00	1.5308	4	17	0
64.00	126.00	52.00	1.5454	4	18	0
64.00	126.00	48.00	1.5430	4	15	0
		MIN THIS CENTRE	1.530			

66.00	126.00	50.00	1.5227	4	15	0
66.00	126.00	52.00	1.5240	4	17	0
66.00	126.00	48.00	1.5706	4	13	0
		MIN THIS CENTRE	1.522			

SF!( 9 ) = 1.522743

68.00	124.00	48.00	1.5549	4	11	0
68.00	124.00	50.00	1.5227	4	15	0
68.00	124.00	52.00	1.5132	4	18	0
68.00	124.00	54.00	1.5297	4	18	0
		MIN THIS CENTRE	1.513			

68.00	126.00	50.00	1.5378	4	13	0
68.00	126.00	52.00	1.5119	4	15	0
68.00	126.00	54.00	1.5085	4	18	0
68.00	126.00	56.00	1.5259	4	19	0
		MIN THIS CENTRE	1.508			

64.00	128.00	52.00	1.5292	4	17	0
64.00	128.00	54.00	1.5453	4	18	0
64.00	128.00	50.00	1.5335	4	15	0
		MIN THIS CENTRE	1.529			

66.00	128.00	52.00	1.5161	4	16	0
66.00	128.00	54.00	1.5215	4	17	0
66.00	128.00	50.00	1.5531	4	13	0
		MIN THIS CENTRE	1.516			

68.00	128.00	52.00	1.5225	4	15	0
68.00	128.00	54.00	1.5057	4	17	0
68.00	128.00	56.00	1.5057	4	18	0
		MIN THIS CENTRE	1.505			

SF!( 9 ) = 1.505717

70.00	126.00	52.00	1.5182	4	13	0
70.00	126.00	54.00	1.4915	4	16	0
70.00	126.00	56.00	1.5011	4	18	0
		MIN THIS CENTRE	1.491			

70.00	128.00	54.00	1.5028	4	15	0
70.00	128.00	56.00	1.4862	4	18	0
70.00	128.00	58.00	1.4978	4	18	0
		MIN THIS CENTRE	1.486			

66.00	130.00	56.00	1.5205	4	18	0
66.00	130.00	58.00	1.5350	4	19	0
66.00	130.00	54.00	1.5133	4	17	0
66.00	130.00	52.00	1.5380	4	15	0
		MIN THIS CENTRE	1.513			

68.00	130.00	56.00	1.5025	4	17	0
68.00	130.00	58.00	1.5044	4	19	0
68.00	130.00	54.00	1.5093	4	15	0
		MIN THIS CENTRE	1.502			

70.00	130.00	56.00	1.4935	4	15	0
70.00	130.00	58.00	1.4833	4	18	0
70.00	130.00	60.00	1.4961	4	19	0
		MIN THIS CENTRE	1.483			

SF!( 9 ) = 1.483264

72.00	128.00	56.00	1.4758	4	16	0
72.00	128.00	58.00	1.4770	4	18	0
72.00	128.00	54.00	1.5076	4	14	0
		MIN THIS CENTRE	1.475			

72.00	130.00	58.00	1.4690	4	17	0
72.00	130.00	60.00	1.4738	4	18	0
72.00	130.00	56.00	1.4930	4	14	0
		MIN THIS CENTRE	1.469			

68.00	132.00	60.00	1.5058	4	19	0
68.00	132.00	62.00	1.5316	4	19	0
68.00	132.00	58.00	1.5012	4	17	0
68.00	132.00	56.00	1.5037	4	16	0

MIN THIS CENTRE 1.501

70.00	132.00	60.00	1.4819	4	18	0
70.00	132.00	62.00	1.4972	4	19	0
70.00	132.00	58.00	1.4888	4	17	0

MIN THIS CENTRE 1.481

72.00	132.00	60.00	1.4653	4	18	0
72.00	132.00	62.00	1.4720	4	19	0
72.00	132.00	58.00	1.4798	4	16	0

MIN THIS CENTRE 1.465

SF!( 9 ) = 1.465326

74.00	130.00	58.00	1.4686	4	16	0
74.00	130.00	60.00	1.4573	4	17	0
74.00	130.00	62.00	1.4548	4	19	0
74.00	130.00	64.00	1.4645	4	24	0

MIN THIS CENTRE 1.454

74.00	132.00	60.00	1.4576	4	16	0
74.00	132.00	62.00	1.4537	4	18	0
74.00	132.00	64.00	1.4529	4	20	0
74.00	132.00	66.00	1.4664	4	24	0

MIN THIS CENTRE 1.452

70.00	134.00	62.00	1.4818	4	19	0
70.00	134.00	64.00	1.5006	4	19	0
70.00	134.00	60.00	1.4864	4	17	0

MIN THIS CENTRE 1.481

72.00	134.00	62.00	1.4635	4	18	0
72.00	134.00	64.00	1.4718	4	19	0
72.00	134.00	60.00	1.4717	4	16	0

MIN THIS CENTRE 1.463

74.00	134.00	62.00	1.4522	4	17	0
74.00	134.00	64.00	1.4518	4	18	0
74.00	134.00	66.00	1.4530	4	20	0

MIN THIS CENTRE 1.451

SF!( 9 ) = 1.451762

76.00	132.00	62.00	1.4424	4	16	0
76.00	132.00	64.00	1.4256	4	19	0
76.00	132.00	66.00	1.4428	4	24	0

MIN THIS CENTRE 1.425

76.00	134.00	64.00	1.4378	4	18	0
76.00	134.00	66.00	1.4240	4	21	0
76.00	134.00	68.00	1.4445	4	24	0

MIN THIS CENTRE 1.423

72.00	136.00	66.00	1.4740	4	19	0
72.00	136.00	68.00	1.4955	4	20	0
72.00	136.00	64.00	1.4631	4	18	0
72.00	136.00	62.00	1.4682	4	18	0
		MIN THIS CENTRE	1.463			

74.00	136.00	66.00	1.4510	4	19	0
74.00	136.00	68.00	1.4553	4	20	0
74.00	136.00	64.00	1.4493	4	18	0
74.00	136.00	62.00	1.4718	4	16	0
		MIN THIS CENTRE	1.449			

76.00	136.00	66.00	1.4352	4	18	0
76.00	136.00	68.00	1.4234	4	22	0
76.00	136.00	70.00	1.4482	4	24	0
		MIN THIS CENTRE	1.423			

SF!( 9 ) = 1.423351

78.00	134.00	66.00	1.4021	4	22	0
78.00	134.00	68.00	1.4225	4	24	0
78.00	134.00	64.00	1.4331	4	16	0
		MIN THIS CENTRE	1.402			

78.00	136.00	68.00	1.4005	4	22	0
78.00	136.00	70.00	1.4229	4	25	0
78.00	136.00	66.00	1.4262	4	16	0
		MIN THIS CENTRE	1.400			

74.00	138.00	70.00	1.4594	4	20	0
74.00	138.00	72.00	1.4839	4	24	0
74.00	138.00	68.00	1.4518	4	19	0
74.00	138.00	66.00	1.4482	4	18	0
74.00	138.00	64.00	1.4634	4	16	0
		MIN THIS CENTRE	1.448			

76.00	138.00	70.00	1.4247	4	22	0
76.00	138.00	72.00	1.4534	4	24	0
76.00	138.00	68.00	1.4341	4	18	0
		MIN THIS CENTRE	1.424			

78.00	138.00	70.00	1.4002	4	22	0
78.00	138.00	72.00	1.4253	4	25	0
78.00	138.00	68.00	1.4225	4	18	0
		MIN THIS CENTRE	1.400			

SF!( 9 ) = 1.400245

80.00	136.00	68.00	1.3861	4	21	0
80.00	136.00	70.00	1.4069	4	25	0
80.00	136.00	66.00	1.4310	4	14	0
		MIN THIS CENTRE	1.386			

80.00	138.00	70.00	1.3842	4	22	0
80.00	138.00	72.00	1.4067	4	26	0

80.00	138.00	68.00	1.4206	4	16	0
		MIN THIS CENTRE		1.384		
76.00	140.00	72.00	1.4280	4	22	0
76.00	140.00	74.00	1.4600	4	24	0
76.00	140.00	70.00	1.4339	4	19	0
		MIN THIS CENTRE		1.427		
78.00	140.00	72.00	1.4008	4	23	0
78.00	140.00	74.00	1.4293	4	25	0
78.00	140.00	70.00	1.4206	4	18	0
		MIN THIS CENTRE		1.400		
80.00	140.00	72.00	1.3836	4	22	0
80.00	140.00	74.00	1.4076	4	26	0
80.00	140.00	70.00	1.4141	4	16	0
		MIN THIS CENTRE		1.383		
SF!( 9 ) =	1.383593					
82.00	138.00	70.00	1.3658	4	23	0
82.00	138.00	72.00	1.3937	4	25	0
82.00	138.00	68.00	1.4327	4	14	0
		MIN THIS CENTRE		1.365		
82.00	140.00	72.00	1.3625	4	24	0
82.00	140.00	74.00	1.3929	4	25	0
82.00	140.00	70.00	1.4220	4	14	0
		MIN THIS CENTRE		1.362		
78.00	142.00	74.00	1.4032	4	23	0
78.00	142.00	76.00	1.4347	4	25	0
78.00	142.00	72.00	1.4201	4	18	0
		MIN THIS CENTRE		1.403		
80.00	142.00	74.00	1.3786	4	24	0
80.00	142.00	76.00	1.4105	4	25	0
80.00	142.00	72.00	1.4110	4	18	0
		MIN THIS CENTRE		1.378		
82.00	142.00	74.00	1.3615	4	24	0
82.00	142.00	76.00	1.3930	4	25	0
82.00	142.00	72.00	1.4123	4	16	0
		MIN THIS CENTRE		1.361		
SF!( 9 ) =	1.361505					
84.00	140.00	72.00	1.3684	4	21	0
84.00	140.00	74.00	1.3783	4	25	0
84.00	140.00	70.00	1.4387	4	12	0
		MIN THIS CENTRE		1.368		
84.00	142.00	74.00	1.3563	4	22	0
84.00	142.00	76.00	1.3767	4	25	0
84.00	142.00	72.00	1.4275	4	14	0
		MIN THIS CENTRE		1.356		

80.00	144.00	76.00	1.3798	4	25	0
80.00	144.00	78.00	1.4148	4	25	0
80.00	144.00	74.00	1.4095	4	18	0
		MIN THIS CENTRE	1.379			

82.00	144.00	76.00	1.3618	4	24	0
82.00	144.00	78.00	1.3939	4	25	0
82.00	144.00	74.00	1.4057	4	16	0
		MIN THIS CENTRE	1.361			

84.00	144.00	76.00	1.3539	4	23	0
84.00	144.00	78.00	1.3762	4	25	0
84.00	144.00	74.00	1.4170	4	14	0
		MIN THIS CENTRE	1.353			

SF!( 9 ) = 1.353943

86.00	142.00	74.00	1.3668	4	19	0
86.00	142.00	76.00	1.3662	4	23	0
86.00	142.00	78.00	1.3519	3	26	0
86.00	142.00	80.00	3.8076	4	33	0
		MIN THIS CENTRE	1.351			

86.00	144.00	76.00	1.3598	4	21	0
86.00	144.00	78.00	1.3639	4	25	0
86.00	144.00	74.00	1.4380	4	12	0
		MIN THIS CENTRE	1.359			

82.00	146.00	78.00	1.3630	4	24	0
82.00	146.00	80.00	1.3965	4	25	0
82.00	146.00	76.00	1.4028	4	18	0
		MIN THIS CENTRE	1.363			

84.00	146.00	78.00	1.3532	4	24	0
84.00	146.00	80.00	1.3764	4	26	0
84.00	146.00	76.00	1.4079	4	16	0
		MIN THIS CENTRE	1.353			

86.00	146.00	78.00	1.3529	4	20	0
86.00	146.00	80.00	1.3630	4	25	0
86.00	146.00	76.00	1.4265	4	14	0
		MIN THIS CENTRE	1.352			

SF!( 3 ) = 1.351876

84.00	140.00	76.00	1.3595	4	25	0
84.00	140.00	78.00	3.7480	4	31	0
84.00	140.00	74.00	1.3783	4	25	0
		MIN THIS CENTRE	1.359			

86.00	140.00	76.00	1.3520	3	25	0
86.00	140.00	78.00	3.8300	4	32	0
86.00	140.00	74.00	1.3709	4	23	0
		MIN THIS CENTRE	1.351			



88.00	140.00	76.00	1.3530	4	25	0
88.00	140.00	78.00	3.9417	4	31	0
88.00	140.00	74.00	1.3801	4	20	0
		MIN THIS CENTRE	1.352			

88.00	142.00	78.00	1.3502	3	26	0
88.00	142.00	80.00	3.9099	4	32	0
88.00	142.00	76.00	1.3723	4	20	0
		MIN THIS CENTRE	1.350			

88.00	144.00	80.00	1.3492	3	27	0
88.00	144.00	82.00	3.8823	4	33	0
88.00	144.00	78.00	1.3649	4	23	0
		MIN THIS CENTRE	1.349			

SF!( 9 ) = 1.349175

90.00	142.00	78.00	1.3594	4	25	0
90.00	142.00	80.00	4.0416	4	31	0
90.00	142.00	76.00	1.3818	4	19	0
		MIN THIS CENTRE	1.359			

90.00	144.00	80.00	1.3539	4	25	0
90.00	144.00	82.00	4.0049	4	30	0
90.00	144.00	78.00	1.3742	4	21	0
		MIN THIS CENTRE	1.353			

86.00	146.00	82.00	1.3540	3	27	0
86.00	146.00	84.00	3.7799	4	33	0
86.00	146.00	80.00	1.3630	4	25	0
		MIN THIS CENTRE	1.353			

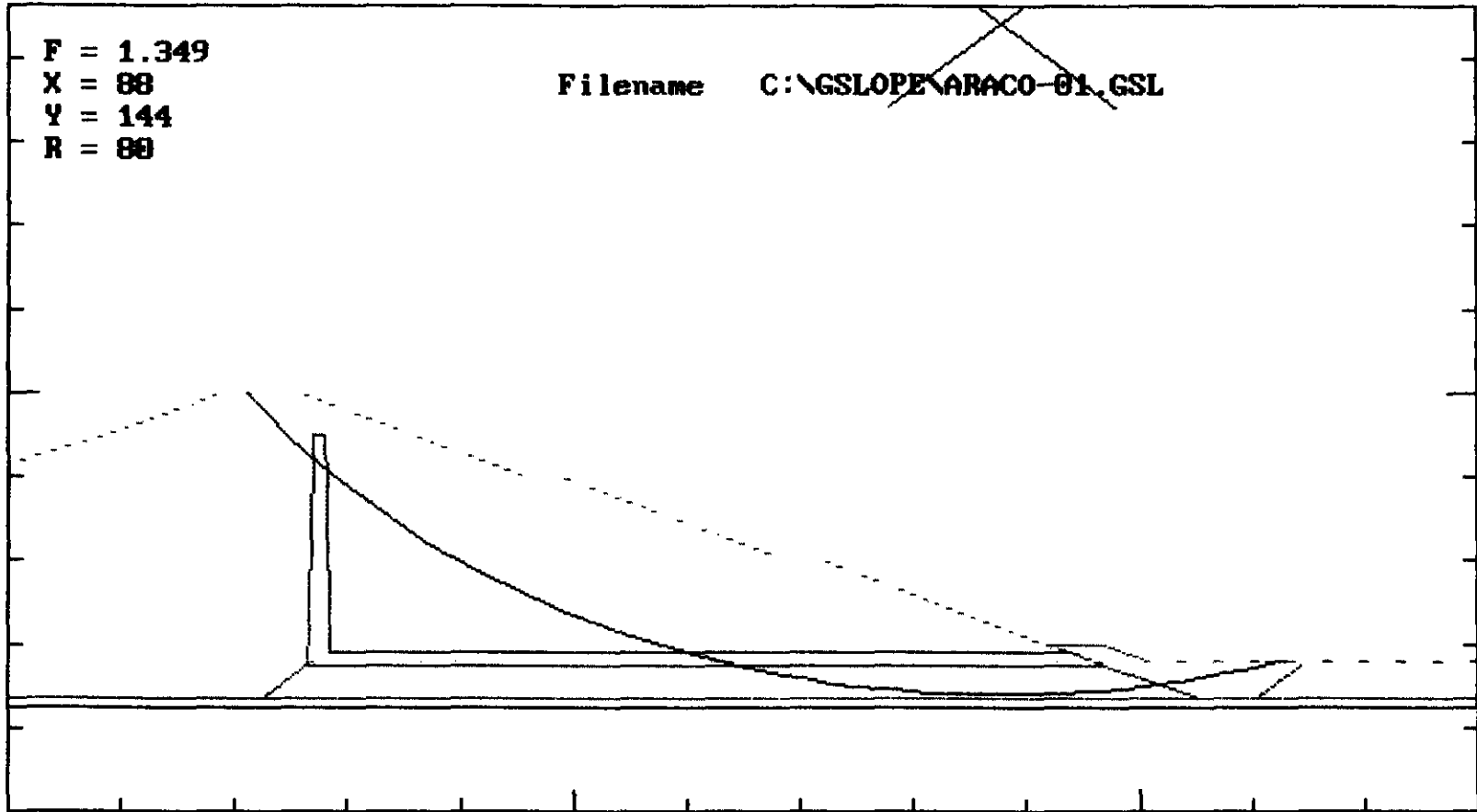
88.00	146.00	82.00	1.3492	3	27	0
88.00	146.00	84.00	3.8600	4	33	0
88.00	146.00	80.00	1.3604	4	23	0
		MIN THIS CENTRE	1.349			

90.00	146.00	82.00	1.3512	3	28	0
90.00	146.00	84.00	3.9727	4	30	0
90.00	146.00	80.00	1.3667	4	21	0
		MIN THIS CENTRE	1.351			

SF!( 5 ) = 1.349175

Minimum Bishop Factor of Safety this run:

88.00	144.00	80.00	1.3492	3	27	0
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DATA FILE NAME..... C:\GSLOPE\ARACO-02.GSL

Job No. Relatorio de Concepcao  
Title Barragem Aracoiaba  
Date 17/11/1997  
Label A Estaca 45 + 0,00  
Label B Talude Jus. - Operacao NA 95

Max Slice Width 10  
No. of Materials 6  
Seismic Acceleration .05  
External Forces 0  
Piezometric Surfaces 1  
Unit Wt. of Pore Fluid 1  
No. of Geogrid Layers 0  
FoS for Geogrid Pullout 0

Material	Unit Wt	Cohesion	Friction Angle	Piezo Surf.	Ru	Interaction Coefficient
1 Enrocamento	2.1	0	38	1	0	0
2 Aterro Compact.	2	2	31	1	0	0
3 Filtro de Areia	1.8	0	30	1	0	0
4 Aluviao	1.8	1.5	28	1	0	0
5 Solo Residual	1.9	1.4	30	1	0	0
6 Rocha	2.2	100	50	1	0	0

Upper Surface of Material # 1 (Enrocamento)

X-Coord	Y-Coord
0	91.4
9	95
19	100
26	100
46	90
49	90
69	80
72	80
92	70
97	70
101	68
115	68
130	68

Upper Surface of Material # 2 (Aterro Compact.)

X-Coord	Y-Coord
0	91.4
9	95
19	100
26	100
46	90 .
49	90
69	80
72	80
92	70

000099

94	69
97	67.5
105.5	63.5
110.5	63.5
115	68
130	68

Upper Surface of Material # 3 (Filtro de Areia)

X-Coord	Y-Coord
0	63.5
22.5	63.5
26.5	67.5
26.9	90
27	95
28	95
28.45	76.5
28.5	69
94	69
97	67.5
105.5	63.5
110.5	63.5
115	68
130	68

Upper Surface of Material # 4 (Aluviao)

X-Coord	Y-Coord
0	63.5
22.5	63.5
26.5	67.5
97	67.5
105.5	63.5
110.5	63.5
130	63.5

Upper Surface of Material # 5 (Solo Residual)

X-Coord	Y-Coord
0	63.5
22.5	63.5
105.5	63.5
110.5	63.5
130	63.5

Upper Surface of Material # 6 (Rocha)

X-Coord	Y-Coord
0	62.5
22	62.5
130	62.5

Piezometric Surface No. 1

X-Coord	Y-Coord
0	95
9	95
26.9	90
28.48	70
99	68
111.5	68
130	68

There are no explicit external forces in the data set.

Geogrid Layer No.	Horizontal Extents X1 <-----> X2	Geogrid Layer Elevation	Strength per unit width
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GSLOPE 3.32

LIMIT EQUILIBRIUM SLOPE STABILITY ANALYSIS

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COBA - Lisboa

Results are for Bishop's Modified Method unless otherwise noted.

File C:\GSLOPE\ARACO-02.GSL Output dated 12-02-1997 at 09:31:34

Material	Unit Wt	Cohesion	Friction Angle	Piezo Surf.	Ru	Interaction Coefficient
1 Enrocamento	2.1	0	38	1	0	0
2 Aterro Compact.	2	2	31	1	0	0
3 Filtro de Areia	1.8	0	30	1	0	0
4 Aluviao	1.8	1.5	28	1	0	0
5 Solo Residual	1.9	1.4	30	1	0	0
6 Rocha	2.2	100	50	1	0	0

X-centre	Y-centre	Radius	Factor of Safety	Iterations	Slices	M Alpha Warnings
50.00	110.00	50.00	7.2454	3	26	0
50.00	110.00	52.00	8.6286	3	27	0
50.00	110.00	48.00	4.6254	4	25	0
50.00	110.00	46.00	2.2609	4	25	0
50.00	110.00	44.00	2.2935	4	24	0
		MIN THIS CENTRE		2.260		
52.00	110.00	50.00	6.9710	3	25	0
52.00	110.00	52.00	8.2895	3	27	0
52.00	110.00	48.00	4.4433	4	26	0
52.00	110.00	46.00	2.1690	4	26	0
52.00	110.00	44.00	2.1992	4	22	0
		MIN THIS CENTRE		2.168		
54.00	110.00	50.00	6.7154	3	27	0
54.00	110.00	52.00	7.9940	3	27	0
54.00	110.00	48.00	4.2839	4	29	0
54.00	110.00	46.00	2.0751	4	23	0
54.00	110.00	44.00	2.1480	4	22	0
		MIN THIS CENTRE		2.075		
50.00	112.00	52.00	7.2495	3	26	0
50.00	112.00	54.00	8.6318	3	27	0
50.00	112.00	50.00	4.6192	4	25	0
50.00	112.00	48.00	2.2453	4	24	0
50.00	112.00	46.00	2.2802	4	24	0
		MIN THIS CENTRE		2.245		
52.00	112.00	52.00	6.9744	3	25	0
52.00	112.00	54.00	8.2916	3	27	0

52.00	112.00	50.00	4.4298	4	27	0
52.00	112.00	48.00	2.1494	4	26	0
52.00	112.00	46.00	2.1748	4	22	0
		MIN THIS CENTRE	2.149			

54.00	112.00	52.00	6.7151	3	27	0
54.00	112.00	54.00	7.9942	3	27	0
54.00	112.00	50.00	4.2740	4	27	0
54.00	112.00	48.00	2.0472	4	23	0
54.00	112.00	46.00	2.1220	4	22	0
		MIN THIS CENTRE	2.047			

50.00	114.00	54.00	7.2629	3	26	0
50.00	114.00	56.00	8.6750	3	28	0
50.00	114.00	52.00	4.6197	3	25	0
50.00	114.00	50.00	2.2347	4	23	0
50.00	114.00	48.00	2.2706	4	24	0
		MIN THIS CENTRE	2.234			

52.00	114.00	54.00	6.9860	3	26	0
52.00	114.00	56.00	8.3041	3	27	0
52.00	114.00	52.00	4.4292	4	27	0
52.00	114.00	50.00	2.1340	4	26	0
52.00	114.00	48.00	2.1898	4	24	0
		MIN THIS CENTRE	2.134			

54.00	114.00	54.00	6.7237	3	26	0
54.00	114.00	56.00	8.0047	3	27	0
54.00	114.00	52.00	4.2726	4	27	0
54.00	114.00	50.00	2.0537	4	26	0
54.00	114.00	48.00	2.1019	4	22	0
		MIN THIS CENTRE	2.053			

SF!( 6 ) = 2.047168

56.00	110.00	46.00	2.0179	4	23	0
56.00	110.00	48.00	4.1355	4	27	0
56.00	110.00	44.00	2.0876	4	24	0
		MIN THIS CENTRE	2.017			

56.00	112.00	48.00	1.9893	4	23	0
56.00	112.00	50.00	4.1513	4	29	0
56.00	112.00	46.00	2.0583	4	24	0
		MIN THIS CENTRE	1.989			

56.00	114.00	50.00	1.9665	4	23	0
56.00	114.00	52.00	4.1458	4	29	0
56.00	114.00	48.00	2.0354	4	24	0
		MIN THIS CENTRE	1.966			

SF!( 9 ) = 1.966477

58.00	112.00	48.00	1.9380	4	23	0
58.00	112.00	50.00	4.0327	4	27	0
58.00	112.00	46.00	2.0209	4	24	0
		MIN THIS CENTRE	1.938			

58.00	114.00	50.00	1.9149	4	23	0
58.00	114.00	52.00	4.0086	4	27	0
58.00	114.00	48.00	1.9949	4	24	0

MIN THIS CENTRE 1.914

54.00	116.00	52.00	2.0415	4	26	0
54.00	116.00	54.00	4.2766	4	27	0
54.00	116.00	50.00	2.0940	4	25	0

MIN THIS CENTRE 2.041

56.00	116.00	52.00	1.9701	4	25	0
56.00	116.00	54.00	4.1458	4	29	0
56.00	116.00	50.00	2.0178	4	24	0

MIN THIS CENTRE 1.970

58.00	116.00	52.00	1.8966	4	23	0
58.00	116.00	54.00	4.0392	4	29	0
58.00	116.00	50.00	1.9746	4	24	0

MIN THIS CENTRE 1.896

SF!( 9 ) = 1.896595

60.00	114.00	50.00	1.8694	4	23	0
60.00	114.00	52.00	3.9334	4	27	0
60.00	114.00	48.00	1.9485	4	24	0

MIN THIS CENTRE 1.869

60.00	116.00	52.00	1.8507	4	23	0
60.00	116.00	54.00	3.9131	4	27	0
60.00	116.00	50.00	1.9274	4	24	0

MIN THIS CENTRE 1.850

56.00	118.00	54.00	1.9613	4	25	0
56.00	118.00	56.00	4.1507	4	29	0
56.00	118.00	52.00	2.0044	4	24	0

MIN THIS CENTRE 1.961

58.00	118.00	54.00	1.8820	4	23	0
58.00	118.00	56.00	4.0300	4	30	0
58.00	118.00	52.00	1.9589	4	24	0

MIN THIS CENTRE 1.882

60.00	118.00	54.00	1.8358	4	23	0
60.00	118.00	56.00	3.9347	4	30	0
60.00	118.00	52.00	1.9106	4	24	0

MIN THIS CENTRE 1.835

SF!( 9 ) = 1.835837

62.00	116.00	52.00	1.8140	4	24	0
62.00	116.00	54.00	3.8569	4	27	0
62.00	116.00	50.00	1.8839	4	23	0

MIN THIS CENTRE 1.814

62.00	118.00	54.00	1.7979	4	24	0
62.00	118.00	56.00	3.8401	4	27	0



62.00	118.00	52.00	1.8714	4	23	0
		MIN THIS CENTRE		1.797		
58.00	120.00	56.00	1.8910	4	25	0
58.00	120.00	58.00	4.0360	4	30	0
58.00	120.00	54.00	1.9469	4	24	0
		MIN THIS CENTRE		1.891		
60.00	120.00	56.00	1.8243	4	23	0
60.00	120.00	58.00	3.9378	4	30	0
60.00	120.00	54.00	1.8976	4	24	0
		MIN THIS CENTRE		1.824		
62.00	120.00	56.00	1.7852	4	24	0
62.00	120.00	58.00	3.8300	4	27	0
62.00	120.00	54.00	1.8575	4	23	0
		MIN THIS CENTRE		1.785		
SF!( 9 ) = 1.785232						
64.00	118.00	54.00	1.7573	4	25	0
64.00	118.00	56.00	3.7949	4	29	0
64.00	118.00	52.00	1.8242	4	23	0
		MIN THIS CENTRE		1.757		
64.00	120.00	56.00	1.7435	4	25	0
64.00	120.00	58.00	3.7820	4	29	0
64.00	120.00	54.00	1.8138	4	23	0
		MIN THIS CENTRE		1.743		
60.00	122.00	58.00	1.8303	4	25	0
60.00	122.00	60.00	3.9448	4	30	0
60.00	122.00	56.00	1.8879	4	24	0
		MIN THIS CENTRE		1.830		
62.00	122.00	58.00	1.7740	4	23	0
62.00	122.00	60.00	3.8580	4	29	0
62.00	122.00	56.00	1.8468	4	23	0
		MIN THIS CENTRE		1.774		
64.00	122.00	58.00	1.7329	4	25	0
64.00	122.00	60.00	3.7753	4	29	0
64.00	122.00	56.00	1.8064	4	23	0
		MIN THIS CENTRE		1.732		
SF!( 9 ) = 1.732854						
66.00	120.00	56.00	1.7090	4	25	0
66.00	120.00	58.00	3.7533	4	30	0
66.00	120.00	54.00	1.7755	4	23	0
		MIN THIS CENTRE		1.709		
66.00	122.00	58.00	1.6994	4	25	0
66.00	122.00	60.00	3.7440	4	29	0
66.00	122.00	56.00	1.7650	4	23	0
		MIN THIS CENTRE		1.699		

62.00	124.00	60.00	1.7623	4	24	0
62.00	124.00	62.00	3.8659	4	31	0
62.00	124.00	58.00	1.8353	4	24	0
		MIN THIS CENTRE	1.762			

64.00	124.00	60.00	1.7247	4	25	0
64.00	124.00	62.00	3.7737	4	30	0
64.00	124.00	58.00	1.7977	4	23	0
		MIN THIS CENTRE	1.724			

66.00	124.00	60.00	1.6904	4	27	0
66.00	124.00	62.00	3.7407	4	29	0
66.00	124.00	58.00	1.7596	4	23	0
		MIN THIS CENTRE	1.690			

SF!( 9 ) = 1.690441

68.00	122.00	58.00	1.6661	4	27	0
68.00	122.00	60.00	3.7408	4	30	0
68.00	122.00	56.00	1.7329	4	23	0
		MIN THIS CENTRE	1.666			

68.00	124.00	60.00	1.6620	4	28	0
68.00	124.00	62.00	3.7310	4	30	0
68.00	124.00	58.00	1.7247	4	23	0
		MIN THIS CENTRE	1.661			

64.00	126.00	62.00	1.7187	4	25	0
64.00	126.00	64.00	3.8072	4	32	0
64.00	126.00	60.00	1.7915	4	23	0
		MIN THIS CENTRE	1.718			

66.00	126.00	62.00	1.6839	4	27	0
66.00	126.00	64.00	3.7419	4	29	0
66.00	126.00	60.00	1.7555	4	23	0
		MIN THIS CENTRE	1.683			

68.00	126.00	62.00	1.6560	4	28	0
68.00	126.00	64.00	3.7263	4	30	0
68.00	126.00	60.00	1.7191	4	23	0
		MIN THIS CENTRE	1.656			

SF!( 9 ) = 1.656014

70.00	124.00	60.00	1.6355	4	27	0
70.00	124.00	62.00	3.7339	4	29	0
70.00	124.00	58.00	1.6895	4	24	0
		MIN THIS CENTRE	1.635			

70.00	126.00	62.00	1.6327	4	26	0
70.00	126.00	64.00	3.7230	4	29	0
70.00	126.00	60.00	1.6845	4	24	0
		MIN THIS CENTRE	1.632			

66.00	128.00	64.00	1.6794	4	27	0
66.00	128.00	66.00	3.7692	4	32	0
66.00	128.00	62.00	1.7504	4	23	0
		MIN THIS CENTRE	1.679			

68.00	128.00	64.00	1.6512	4	28	0
68.00	128.00	66.00	3.7225	4	29	0
68.00	128.00	62.00	1.7173	4	23	0
		MIN THIS CENTRE	1.651			

70.00	128.00	64.00	1.6334	4	27	0
70.00	128.00	66.00	3.7171	4	29	0
70.00	128.00	62.00	1.6795	4	24	0
		MIN THIS CENTRE	1.633			

SF!( 6 ) = 1.632708

72.00	124.00	60.00	1.6151	4	28	0
72.00	124.00	62.00	3.7364	4	28	0
72.00	124.00	58.00	1.6637	4	25	0
		MIN THIS CENTRE	1.615			

72.00	126.00	62.00	1.6114	4	28	0
72.00	126.00	64.00	3.7232	4	28	0
72.00	126.00	60.00	1.6562	4	26	0
		MIN THIS CENTRE	1.611			

72.00	128.00	64.00	1.6074	4	28	0
72.00	128.00	66.00	3.7152	4	28	0
72.00	128.00	62.00	1.6518	4	26	0
		MIN THIS CENTRE	1.607			

SF!( 9 ) = 1.607393

74.00	126.00	62.00	1.5938	4	28	0
74.00	126.00	64.00	3.7358	4	30	0
74.00	126.00	60.00	1.6352	4	25	0
		MIN THIS CENTRE	1.593			

74.00	128.00	64.00	1.5903	4	28	0
74.00	128.00	66.00	3.7161	4	31	0
74.00	128.00	62.00	1.6325	4	26	0
		MIN THIS CENTRE	1.590			

70.00	130.00	66.00	1.6299	4	28	0
70.00	130.00	68.00	3.7155	4	29	0
70.00	130.00	64.00	1.6772	4	26	0
		MIN THIS CENTRE	1.629			

72.00	130.00	66.00	1.6064	4	28	0
72.00	130.00	68.00	3.7080	4	28	0
72.00	130.00	64.00	1.6504	4	27	0
		MIN THIS CENTRE	1.606			

74.00	130.00	66.00	1.5884	4	28	0
74.00	130.00	68.00	3.7079	4	31	0

74.00	130.00	64.00	1.6285	4	26	0
		MIN THIS CENTRE		1.588		
SF!( 9 ) = 1.588401						
76.00	128.00	64.00	1.5758	4	28	0
76.00	128.00	66.00	3.7417	4	31	0
76.00	128.00	62.00	1.6158	4	25	0
		MIN THIS CENTRE		1.575		
76.00	130.00	66.00	1.5710	4	27	0
76.00	130.00	68.00	3.7263	4	31	0
76.00	130.00	64.00	1.6150	4	25	0
		MIN THIS CENTRE		1.571		
72.00	132.00	68.00	1.6081	4	28	0
72.00	132.00	70.00	3.7050	4	28	0
72.00	132.00	66.00	1.6488	4	27	0
		MIN THIS CENTRE		1.608		
74.00	132.00	68.00	1.5867	4	28	0
74.00	132.00	70.00	3.7034	4	31	0
74.00	132.00	66.00	1.6270	4	26	0
		MIN THIS CENTRE		1.586		
76.00	132.00	68.00	1.5686	4	27	0
76.00	132.00	70.00	3.7164	4	31	0
76.00	132.00	66.00	1.6150	4	26	0
		MIN THIS CENTRE		1.568		
SF!( 9 ) = 1.568586						
78.00	130.00	66.00	1.5527	4	26	0
78.00	130.00	68.00	3.7675	4	32	0
78.00	130.00	64.00	1.6041	4	25	0
		MIN THIS CENTRE		1.552		
78.00	132.00	68.00	1.5499	4	27	0
78.00	132.00	70.00	3.7520	4	32	0
78.00	132.00	66.00	1.6037	4	25	0
		MIN THIS CENTRE		1.549		
74.00	134.00	70.00	1.5871	4	28	0
74.00	134.00	72.00	3.6999	4	31	0
74.00	134.00	68.00	1.6281	4	26	0
		MIN THIS CENTRE		1.587		
76.00	134.00	70.00	1.5675	4	27	0
76.00	134.00	72.00	3.7125	4	31	0
76.00	134.00	68.00	1.6141	4	26	0
		MIN THIS CENTRE		1.567		
78.00	134.00	70.00	1.5470	4	28	0
78.00	134.00	72.00	3.7398	4	32	0
78.00	134.00	68.00	1.5987	4	26	0
		MIN THIS CENTRE		1.547		

SF!( 9 ) = 1.547033

80.00	132.00	68.00	1.5320	4	27	0
80.00	132.00	70.00	3.8039	4	32	0
80.00	132.00	66.00	1.5877	4	26	0
		MIN THIS CENTRE	1.531			

80.00	134.00	70.00	1.5311	4	28	0
80.00	134.00	72.00	3.7856	4	32	0
80.00	134.00	68.00	1.5865	4	27	0
		MIN THIS CENTRE	1.531			

76.00	136.00	72.00	1.5667	4	27	0
76.00	136.00	74.00	3.7112	4	31	0
76.00	136.00	70.00	1.6147	4	26	0
		MIN THIS CENTRE	1.566			

78.00	136.00	72.00	1.5465	4	28	0
78.00	136.00	74.00	3.7327	4	32	0
78.00	136.00	70.00	1.5994	4	27	0
		MIN THIS CENTRE	1.546			

80.00	136.00	72.00	1.5307	4	28	0
80.00	136.00	74.00	3.7736	4	32	0
80.00	136.00	70.00	1.5866	4	27	0
		MIN THIS CENTRE	1.530			

SF!( 9 ) = 1.530713

82.00	134.00	70.00	1.5176	4	26	0
82.00	134.00	72.00	3.8532	4	33	0
82.00	134.00	68.00	1.5798	4	25	0
		MIN THIS CENTRE	1.517			

82.00	136.00	72.00	1.5182	4	27	0
82.00	136.00	74.00	3.8314	4	33	0
82.00	136.00	70.00	1.5774	4	27	0
		MIN THIS CENTRE	1.518			

78.00	138.00	74.00	1.5471	4	28	0
78.00	138.00	76.00	3.7305	4	32	0
78.00	138.00	72.00	1.5999	4	27	0
		MIN THIS CENTRE	1.547			

80.00	138.00	74.00	1.5299	4	28	0
80.00	138.00	76.00	3.7642	4	32	0
80.00	138.00	72.00	1.5875	4	28	0
		MIN THIS CENTRE	1.529			

82.00	138.00	74.00	1.5188	4	28	0
82.00	138.00	76.00	3.8156	4	33	0
82.00	138.00	72.00	1.5768	4	27	0
		MIN THIS CENTRE	1.518			

SF!( 3 ) = 1.517603

80.00	132.00	68.00	1.5320	4	27	0
80.00	132.00	70.00	3.8039	4	32	0
80.00	132.00	66.00	1.5877	4	26	0

MIN THIS CENTRE 1.531

82.00	132.00	68.00	1.5182	4	25	0
82.00	132.00	70.00	3.8783	4	31	0
82.00	132.00	66.00	1.5856	4	25	0

MIN THIS CENTRE 1.518

84.00	132.00	68.00	1.5119	4	25	0
84.00	132.00	70.00	3.9708	4	32	0
84.00	132.00	66.00	1.5890	4	21	0

MIN THIS CENTRE 1.511

84.00	134.00	70.00	1.5092	4	25	0
84.00	134.00	72.00	3.9395	4	32	0
84.00	134.00	68.00	1.5814	4	22	0

MIN THIS CENTRE 1.509

84.00	136.00	72.00	1.5083	4	27	0
84.00	136.00	74.00	3.9120	4	32	0
84.00	136.00	70.00	1.5720	4	24	0

MIN THIS CENTRE 1.508

SF!( 9 ) = 1.508287

86.00	134.00	70.00	1.5146	4	26	0
86.00	134.00	72.00	4.0515	4	30	0
86.00	134.00	68.00	1.5818	4	19	0

MIN THIS CENTRE 1.514

86.00	136.00	72.00	1.5074	4	26	0
86.00	136.00	74.00	4.0163	4	34	0
86.00	136.00	70.00	1.5762	4	23	0

MIN THIS CENTRE 1.507

82.00	138.00	74.00	1.5188	4	28	0
82.00	138.00	76.00	3.8156	4	33	0
82.00	138.00	72.00	1.5768	4	27	0

MIN THIS CENTRE 1.518

84.00	138.00	74.00	1.5088	4	27	0
84.00	138.00	76.00	3.8887	4	33	0
84.00	138.00	72.00	1.5669	4	24	0

MIN THIS CENTRE 1.508

86.00	138.00	74.00	1.5056	4	26	0
86.00	138.00	76.00	3.9858	4	34	0
86.00	138.00	72.00	1.5694	4	23	0

MIN THIS CENTRE 1.505

SF!( 9 ) = 1.50555

88.00	136.00	72.00	1.5197	4	25	0
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88.00	136.00	74.00	4.1505	4	31	0
88.00	136.00	70.00	1.5844	4	15	0
		MIN THIS CENTRE	1.519			

88.00	138.00	74.00	1.5141	4	27	0
88.00	138.00	76.00	4.1088	4	32	0
88.00	138.00	72.00	1.5790	4	18	0
		MIN THIS CENTRE	1.514			

84.00	140.00	76.00	1.5104	4	27	0
84.00	140.00	78.00	3.8691	4	33	0
84.00	140.00	74.00	1.5650	4	27	0
		MIN THIS CENTRE	1.510			

86.00	140.00	76.00	1.5051	4	27	0
86.00	140.00	78.00	3.9588	4	34	0
86.00	140.00	74.00	1.5611	4	25	0
		MIN THIS CENTRE	1.505			

88.00	140.00	76.00	1.5079	4	27	0
88.00	140.00	78.00	4.0736	4	33	0
88.00	140.00	74.00	1.5735	4	22	0
		MIN THIS CENTRE	1.507			

SF!( 8 ) = 1.505137

84.00	142.00	78.00	1.5119	4	28	0
84.00	142.00	80.00	3.8548	4	33	0
84.00	142.00	76.00	1.5646	4	27	0
		MIN THIS CENTRE	1.511			

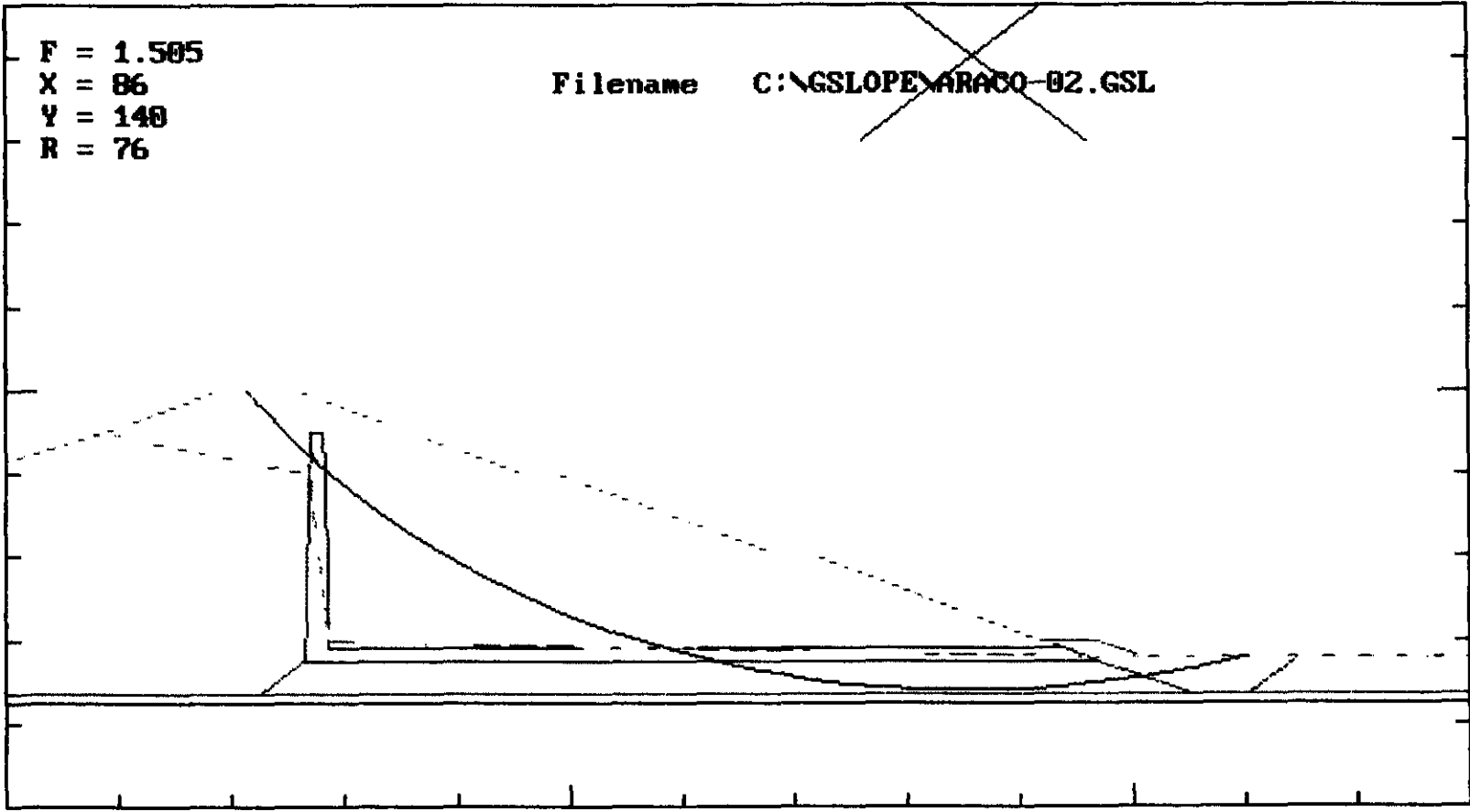
86.00	142.00	78.00	1.5063	4	28	0
86.00	142.00	80.00	3.9363	4	35	0
86.00	142.00	76.00	1.5570	4	25	0
		MIN THIS CENTRE	1.506			

88.00	142.00	78.00	1.5066	4	28	0
88.00	142.00	80.00	4.0427	4	34	0
88.00	142.00	76.00	1.5675	4	22	0
		MIN THIS CENTRE	1.506			

SF!( 5 ) = 1.505137

Minimum Bishop Factor of Safety this run:

86.00	140.00	76.00	1.5051	4	27	0
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DATA FILE NAME..... C:\GSLOPE\ARACO-04.GSL

Job No. Relatorio de Concepcao  
Title Barragem Aracoiaba  
Date 17/11/1997  
Label A Estaca 45 + 0,00  
Label B Talude Mon. - Rebaixamento

Max Slice Width 10  
No. of Materials 7  
Seismic Acceleration .05  
External Forces 0  
Piezometric Surfaces 1  
Unit Wt. of Pore Fluid 1  
No. of Geogrid Layers 0  
FoS for Geogrid Pullout 0

Material	Unit Wt	Cohesion	Friction Angle	Piezo Surf.	Ru	Interaction Coefficient
1 Enchimento	1.9	0	20	1	0	0
2 Ensecadeira	2	1	28	1	0	0
3 Aterro Compact.	2	2	31	1	0	0
4 Filtro de Areia	1.8	0	30	1	0	0
5 Aluviao	1.8	1.5	28	1	0	0
6 Solo Residual	1.9	1.4	30	1	0	0
7 Rocha	2.2	100	50	1	0	0

Upper Surface of Material # 1 (Enchimento)

X-Coord	Y-Coord
4	90
24	100
31	100
41	95
91	75
120	75
125	75
134	69
150	69

Upper Surface of Material # 2 (Ensecadeira)

X-Coord	Y-Coord
4	90
24	100
31	100
41	95
91	75
106	69
111	69
120	75
125	75
134	69
150	69

Upper Surface of Material # 3 (Aterro Compact.)

X-Coord	Y-Coord
4	90
24	100
31	100
41	95
91	75
106	69
111	69
134	69
150	69

Upper Surface of Material # 4 (Filtro de Areia)

X-Coord	Y-Coord
4	69
21.5	69
22	95
23	95
23.5	67.5
27.5	63.5
28.5	62.5
53.5	62.5
54.5	63.5
58.5	67.5
105	67.5
106	69
111	69
134	69
150	69

Upper Surface of Material # 5 (Aluviao)

X-Coord	Y-Coord
4	67.5
23.5	67.5
27.5	63.5
28.5	62.5
53.5	62.5
54.5	63.5
58.5	67.5
105	67.5
106	69
111	69
134	69
150	69

Upper Surface of Material # 6 (Solo Residual)

X-Coord	Y-Coord
4	63.5
27.5	63.5
28.5	62.5
53.5	62.5
54.5	63.5
150	63.5

Upper Surface of Material # 7 (Rocha)

X-Coord	Y-Coord
4	62.5
28.5	62.5
100	62.5

150 62.5

Piezometric Surface No.	1
X-Coord	Y-Coord
4	69
21.55	70
23.1	90
41	95
91	75
120	75
125	75
150	75

There are no explicit external forces in the data set.

Geogrid Layer No.	Horizontal Extents X1 <-----> X2	Geogrid Layer Elevation	Strength per unit width
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GSLOPE 3.32

LIMIT EQUILIBRIUM SLOPE STABILITY ANALYSIS

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Results are for Bishop's Modified Method unless otherwise noted.

File C:\GSLOPE\ARACO-04.GSL Output dated 12-01-1997 at 15:33:52

Material	Unit Wt	Cohesion	Friction Angle	Piezo Surf.	Ru	Interaction Coefficient
1 Enchimento	1.9	0	20	1	0	0
2 Ensecadeira	2	1	28	1	0	0
3 Aterro Compact.	2	2	31	1	0	0
4 Filtro de Areia	1.8	0	30	1	0	0
5 Aluviao	1.8	1.5	28	1	0	0
6 Solo Residual	1.9	1.4	30	1	0	0
7 Rocha	2.2	100	50	1	0	0

X-centre	Y-centre	Radius	Factor of Safety	Iterations	Slices	M Alpha Warnings
60.00	110.00	50.00	5.9583	3	23	0
60.00	110.00	52.00	7.3282	3	25	0
60.00	110.00	48.00	3.6412	4	24	0
60.00	110.00	46.00	1.5092	4	21	0
60.00	110.00	44.00	1.4846	4	21	0
60.00	110.00	42.00	1.4916	4	19	0
		MIN THIS CENTRE	1.484			
62.00	110.00	50.00	5.8189	3	23	0
62.00	110.00	52.00	7.0689	3	25	0
62.00	110.00	48.00	3.5197	4	26	0
62.00	110.00	46.00	1.4156	4	24	0
62.00	110.00	44.00	1.4044	3	21	0
62.00	110.00	42.00	1.4332	4	18	0
		MIN THIS CENTRE	1.404			
64.00	110.00	50.00	5.7082	3	24	0
64.00	110.00	52.00	6.9519	3	26	0
64.00	110.00	48.00	3.4337	4	26	0
64.00	110.00	46.00	1.3536	3	23	0
64.00	110.00	44.00	1.3422	3	21	0
64.00	110.00	42.00	1.3690	3	15	0
		MIN THIS CENTRE	1.342			
60.00	112.00	52.00	6.0342	3	24	0
60.00	112.00	54.00	7.3743	3	25	0
60.00	112.00	50.00	3.6452	4	24	0
60.00	112.00	48.00	1.5133	4	21	0
60.00	112.00	46.00	1.4868	4	21	0
60.00	112.00	44.00	1.4898	4	19	0

MIN THIS CENTRE 1.486

62.00	112.00	52.00	5.8323	3	23	0
62.00	112.00	54.00	7.1588	3	26	0
62.00	112.00	50.00	3.5239	4	26	0
62.00	112.00	48.00	1.4180	4	24	0
62.00	112.00	46.00	1.4027	3	21	0
62.00	112.00	44.00	1.4230	4	19	0

MIN THIS CENTRE 1.402

64.00	112.00	52.00	5.7149	3	25	0
64.00	112.00	54.00	6.9649	3	26	0
64.00	112.00	50.00	3.4357	4	26	0
64.00	112.00	48.00	1.3522	3	23	0
64.00	112.00	46.00	1.3334	3	23	0
64.00	112.00	44.00	1.3620	3	17	0

MIN THIS CENTRE 1.333

60.00	114.00	54.00	6.0744	3	24	0
60.00	114.00	56.00	7.4300	3	26	0
60.00	114.00	52.00	3.6577	4	24	0
60.00	114.00	50.00	1.5191	4	23	0
60.00	114.00	48.00	1.4926	4	21	0
60.00	114.00	46.00	1.4933	4	19	0

MIN THIS CENTRE 1.492

62.00	114.00	54.00	5.8570	3	23	0
62.00	114.00	56.00	7.2057	3	26	0
62.00	114.00	52.00	3.5358	4	26	0
62.00	114.00	50.00	1.4244	4	24	0
62.00	114.00	48.00	1.4060	4	21	0
62.00	114.00	46.00	1.4221	4	19	0

MIN THIS CENTRE 1.405

64.00	114.00	54.00	5.7298	3	25	0
64.00	114.00	56.00	6.9904	3	26	0
64.00	114.00	52.00	3.4458	4	26	0
64.00	114.00	50.00	1.3559	3	23	0
64.00	114.00	48.00	1.3321	3	23	0
64.00	114.00	46.00	1.3519	3	20	0

MIN THIS CENTRE 1.332

SF!( 9 ) = 1.332073

66.00	112.00	46.00	1.2819	3	20	0
66.00	112.00	48.00	1.2936	2	23	0
66.00	112.00	44.00	1.3132	3	14	0

MIN THIS CENTRE 1.281

66.00	114.00	48.00	1.2735	3	23	0
66.00	114.00	50.00	1.2949	2	23	0
66.00	114.00	46.00	1.3085	2	16	0

MIN THIS CENTRE 1.273

62.00	116.00	50.00	1.4125	4	21	0
62.00	116.00	52.00	1.4336	4	24	0
62.00	116.00	48.00	1.4140	4	20	0

MIN THIS CENTRE 1.412

64.00	116.00	50.00	1.3354	3	23	0
64.00	116.00	52.00	1.3632	3	23	0
64.00	116.00	48.00	1.3520	3	20	0
		MIN THIS CENTRE	1.335			

66.00	116.00	50.00	1.2751	3	23	0
66.00	116.00	52.00	1.3004	1	23	0
66.00	116.00	48.00	1.3053	2	20	0
		MIN THIS CENTRE	1.275			

SF!( 6 ) = 1.273549

68.00	112.00	46.00	1.2319	3	18	0
68.00	112.00	48.00	1.2555	3	20	0
68.00	112.00	44.00	1.2734	3	15	0
		MIN THIS CENTRE	1.231			

68.00	114.00	48.00	1.2305	3	20	0
68.00	114.00	50.00	1.2501	3	23	0
68.00	114.00	46.00	1.2653	3	15	0
		MIN THIS CENTRE	1.230			

68.00	116.00	50.00	1.2273	3	24	0
68.00	116.00	52.00	1.2530	3	23	0
68.00	116.00	48.00	1.2606	3	18	0
		MIN THIS CENTRE	1.227			

SF!( 9 ) = 1.227261

70.00	114.00	48.00	1.1986	4	16	0
70.00	114.00	50.00	1.2242	3	20	0
70.00	114.00	46.00	1.2337	3	15	0
		MIN THIS CENTRE	1.198			

70.00	116.00	50.00	1.1966	4	19	0
70.00	116.00	52.00	1.2155	4	25	0
70.00	116.00	48.00	1.2268	4	15	0
		MIN THIS CENTRE	1.196			

66.00	118.00	52.00	1.2798	3	23	0
66.00	118.00	54.00	1.3091	2	23	0
66.00	118.00	50.00	1.3011	2	22	0
		MIN THIS CENTRE	1.279			

68.00	118.00	52.00	1.2273	3	24	0
68.00	118.00	54.00	1.2589	3	24	0
68.00	118.00	50.00	1.2616	3	19	0
		MIN THIS CENTRE	1.227			

70.00	118.00	52.00	1.1995	4	20	0
70.00	118.00	54.00	1.2175	4	25	0
70.00	118.00	50.00	1.2243	4	17	0
		MIN THIS CENTRE	1.199			

SF!( 6 ) = 1.196629

72.00	114.00	48.00	1.1739	4	16	0
72.00	114.00	50.00	1.1932	4	18	0
72.00	114.00	46.00	1.2075	4	14	0
		MIN THIS CENTRE	1.173			

72.00	116.00	50.00	1.1694	4	17	0
72.00	116.00	52.00	1.1931	4	21	0
72.00	116.00	48.00	1.2022	4	15	0
		MIN THIS CENTRE	1.169			

72.00	118.00	52.00	1.1680	4	19	0
72.00	118.00	54.00	1.1952	4	22	0
72.00	118.00	50.00	1.1997	4	15	0
		MIN THIS CENTRE	1.167			

SF!( 9 ) = 1.16799

74.00	116.00	50.00	1.1535	4	17	0
74.00	116.00	52.00	1.1707	4	19	0
74.00	116.00	48.00	1.1871	4	13	0
		MIN THIS CENTRE	1.153			

74.00	118.00	52.00	1.1511	4	17	0
74.00	118.00	54.00	1.1688	4	22	0
74.00	118.00	50.00	1.1810	4	15	0
		MIN THIS CENTRE	1.151			

70.00	120.00	54.00	1.1974	4	23	0
70.00	120.00	56.00	1.2237	3	25	0
70.00	120.00	52.00	1.2251	4	18	0
		MIN THIS CENTRE	1.197			

72.00	120.00	54.00	1.1702	4	20	0
72.00	120.00	56.00	1.1934	4	26	0
72.00	120.00	52.00	1.1977	4	15	0
		MIN THIS CENTRE	1.170			

74.00	120.00	54.00	1.1506	4	18	0
74.00	120.00	56.00	1.1710	4	23	0
74.00	120.00	52.00	1.1797	4	16	0
		MIN THIS CENTRE	1.150			

SF!( 9 ) = 1.150601

76.00	118.00	52.00	1.1416	4	18	0
76.00	118.00	54.00	1.1523	4	20	0
76.00	118.00	50.00	1.1765	4	14	0
		MIN THIS CENTRE	1.141			

76.00	120.00	54.00	1.1413	4	18	0
76.00	120.00	56.00	1.1517	4	21	0
76.00	120.00	52.00	1.1683	4	15	0
		MIN THIS CENTRE	1.141			

72.00	122.00	56.00	1.1748	4	24	0
72.00	122.00	58.00	1.1982	4	26	0
72.00	122.00	54.00	1.1987	4	18	0
		MIN THIS CENTRE	1.174			

74.00	122.00	56.00	1.1522	4	20	0
74.00	122.00	58.00	1.1727	4	27	0
74.00	122.00	54.00	1.1792	4	16	0
		MIN THIS CENTRE	1.152			

76.00	122.00	56.00	1.1401	4	19	0
76.00	122.00	58.00	1.1531	4	23	0
76.00	122.00	54.00	1.1656	4	16	0
		MIN THIS CENTRE	1.140			

SF!( 9 ) = 1.14013

78.00	120.00	54.00	1.1349	4	18	0
78.00	120.00	56.00	1.1430	4	21	0
78.00	120.00	52.00	1.1739	4	14	0
		MIN THIS CENTRE	1.134			

78.00	122.00	56.00	1.1327	4	19	0
78.00	122.00	58.00	1.1414	4	21	0
78.00	122.00	54.00	1.1646	4	14	0
		MIN THIS CENTRE	1.132			

74.00	124.00	58.00	1.1571	4	22	0
74.00	124.00	60.00	1.1747	4	27	0
74.00	124.00	56.00	1.1797	4	17	0
		MIN THIS CENTRE	1.157			

76.00	124.00	58.00	1.1410	4	21	0
76.00	124.00	60.00	1.1581	4	24	0
76.00	124.00	56.00	1.1665	4	16	0
		MIN THIS CENTRE	1.141			

78.00	124.00	58.00	1.1336	4	19	0
78.00	124.00	60.00	1.1423	4	23	0
78.00	124.00	56.00	1.1597	4	17	0
		MIN THIS CENTRE	1.133			

SF!( 6 ) = 1.132745

80.00	120.00	54.00	1.1417	4	17	0
80.00	120.00	56.00	1.1417	4	20	0
80.00	120.00	58.00	3.4914	4	24	0
		MIN THIS CENTRE	1.141			

80.00	122.00	56.00	1.1321	4	17	0
80.00	122.00	58.00	1.1394	4	21	0
80.00	122.00	54.00	1.1708	4	16	0
		MIN THIS CENTRE	1.132			

80.00	124.00	58.00	1.1280	4	20	0
80.00	124.00	60.00	1.1385	4	21	0



80.00	124.00	56.00	1.1682	4	16	0
		MIN THIS CENTRE		1.128		

SF!( 9 ) = 1.128001

82.00	122.00	56.00	1.1426	4	19	0
82.00	122.00	58.00	1.1459	4	19	0
82.00	122.00	54.00	1.1851	4	14	0
		MIN THIS CENTRE		1.142		

82.00	124.00	58.00	1.1369	4	19	0
82.00	124.00	60.00	1.1414	4	20	0
82.00	124.00	56.00	1.1783	4	14	0
		MIN THIS CENTRE		1.136		

78.00	126.00	60.00	1.1331	4	20	0
78.00	126.00	62.00	1.1456	4	24	0
78.00	126.00	58.00	1.1587	4	17	0
		MIN THIS CENTRE		1.133		

80.00	126.00	60.00	1.1273	4	20	0
80.00	126.00	62.00	1.1378	4	22	0
80.00	126.00	58.00	1.1602	4	17	0
		MIN THIS CENTRE		1.127		

82.00	126.00	60.00	1.1300	4	20	0
82.00	126.00	62.00	1.1406	4	21	0
82.00	126.00	58.00	1.1696	4	16	0
		MIN THIS CENTRE		1.129		

SF!( 8 ) = 1.127316

78.00	128.00	62.00	1.1353	4	22	0
78.00	128.00	64.00	1.1516	4	26	0
78.00	128.00	60.00	1.1607	4	18	0
		MIN THIS CENTRE		1.135		

80.00	128.00	62.00	1.1289	4	20	0
80.00	128.00	64.00	1.1393	4	24	0
80.00	128.00	60.00	1.1569	4	18	0
		MIN THIS CENTRE		1.128		

82.00	128.00	62.00	1.1272	4	20	0
82.00	128.00	64.00	1.1405	4	21	0
82.00	128.00	60.00	1.1670	4	16	0
		MIN THIS CENTRE		1.127		

SF!( 9 ) = 1.127202

84.00	126.00	60.00	1.1452	4	19	0
84.00	126.00	62.00	1.1488	4	20	0
84.00	126.00	58.00	1.1905	4	15	0
		MIN THIS CENTRE		1.145		

84.00	128.00	62.00	1.1395	4	19	0
84.00	128.00	64.00	1.1464	4	21	0
84.00	128.00	60.00	1.1771	4	17	0

MIN THIS CENTRE 1.139

80.00	130.00	64.00	1.1295	4	21	0
80.00	130.00	66.00	1.1429	4	25	0
80.00	130.00	62.00	1.1568	4	18	0

MIN THIS CENTRE 1.129

82.00	130.00	64.00	1.1277	4	20	0
82.00	130.00	66.00	1.1392	4	24	0
82.00	130.00	62.00	1.1606	4	17	0

MIN THIS CENTRE 1.127

84.00	130.00	64.00	1.1343	4	21	0
84.00	130.00	66.00	1.1612	4	21	0
84.00	130.00	62.00	1.1731	4	17	0

MIN THIS CENTRE 1.134

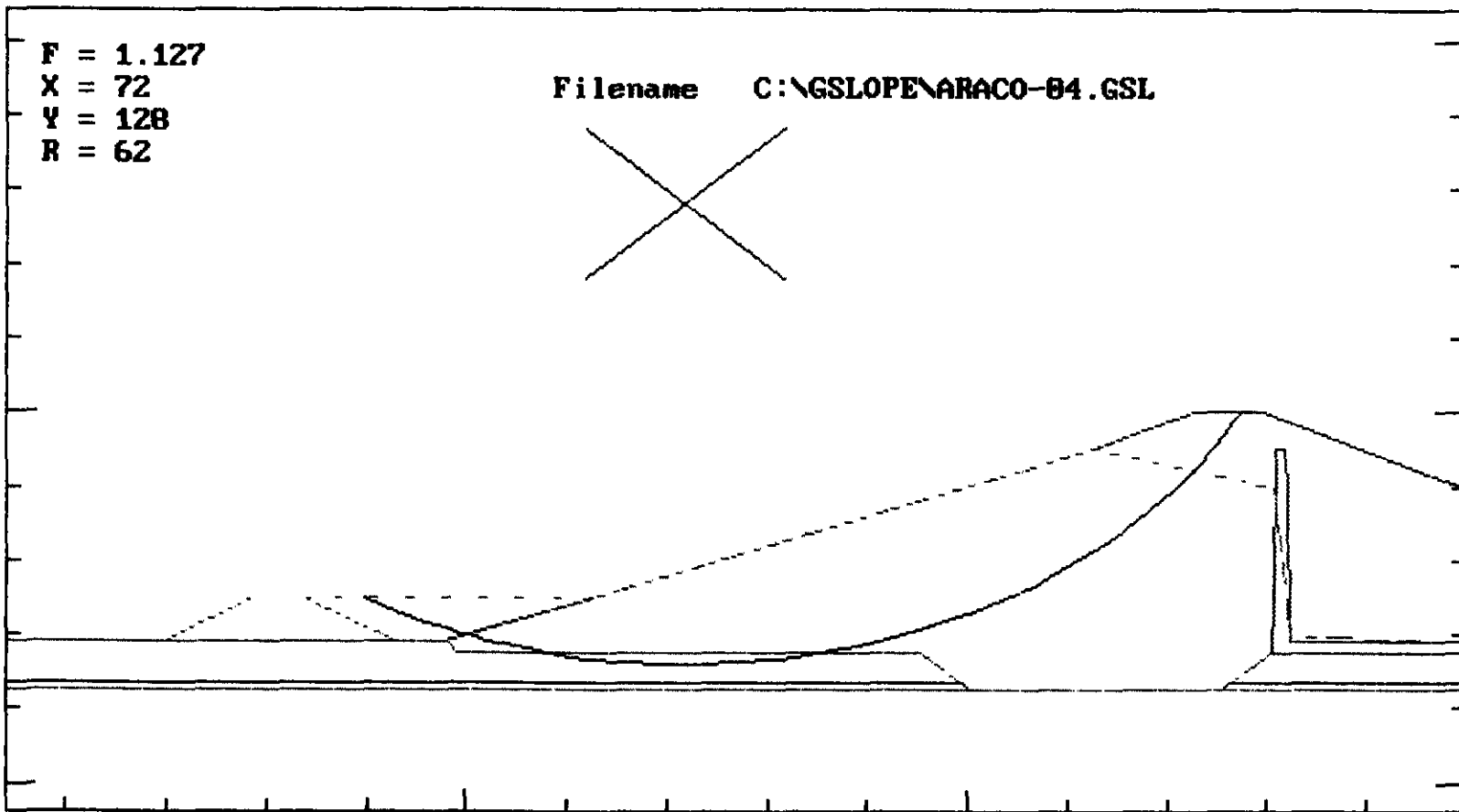
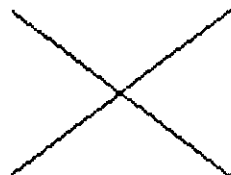
SF!( 5 ) = 1.127202

Minimum Bishop Factor of Safety this run:

82.00	128.00	62.00	1.1272	4	20	0
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F = 1.127  
X = 72  
Y = 128  
R = 62

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000123

## **6 - ESTUDOS DE OTIMIZAÇÃO**

Para os estudos de otimização foram calculados os volumes das estruturas de terra e de concreto para os níveis d'água de operação de 95,00, 90,00 e 85,00 conforme os cálculos apresentados a seguir

### **6.1 - Obras de Terra**

Para os cálculos dos volumes das obras de terra foram utilizadas planilhas de cálculo, com algumas simplificações

As três alternativas foram calculadas com base na Memória de Cálculo da Alternativa 1, examinada com detalhes anteriormente. Os volumes foram determinados com aproximações percentuais relativos

### **6.2 - Vertedouro e Canais**

Os cálculos para dimensionamento do vertedouro, considerado de soleira livre, das três alternativas são apresentados a seguir

Os volumes de escavação e de concreto a serem utilizados foram determinados a partir de croquis levantados nos locais de implantação dos mesmos

**ANEXO 6.1  
ESTUDOS HIDRÁULICOS**

\*\*\*\*\*  
 \*  
 \* AMORTECIMENTO DA ONDA DE CHEIA \*  
 \*\*\*\*\*

NIVEL DE PLENO ARMAZENAMENTO(m)..... 85.00  
 COTA DA SOLEIRA DESCARREGADORA(m)..... 85.00  
 COMPRIMENTO DA CRISTA(m).....120.00  
 COEFICIENTE DE VAZ||O..... .400

CURVA DE VOLUMES:V=a(Z-c)\*\*b  
 PARAMETRO a..... .0121610  
 PARAMETRO b..... 2.8147  
 PARAMETRO c..... 65.00

TEMPO (h)	Qafl (m3/s)	Vmed (hm3)	Vtot (hm3)	Zint (m)	Qdes (m3/s)	Vdes (hm3)	Vfin (hm3)	Zfin (m)
.0	.0	.00	.00	85.00	.0	.00	55.84	.00
1.0	40.0	.07	55.91	85.01	.2	.00	55.91	85.01
2.0	145.0	.33	56.24	85.05	2.5	.01	56.24	85.05
3.0	290.0	.78	57.02	85.15	12.2	.04	56.97	85.14
4.0	370.0	1.19	58.16	85.29	33.5	.12	58.04	85.28
5.0	680.0	1.89	59.93	85.51	77.1	.28	59.65	85.47
6.0	900.0	2.84	62.50	85.82	156.9	.56	61.93	85.75
7.0	1100.0	3.60	65.53	86.17	269.1	.97	64.56	86.06
8.0	1280.0	4.28	68.85	86.54	408.2	1.47	67.38	86.38
9.0	1350.0	4.73	72.11	86.90	557.8	2.01	70.11	86.68
10.0	1380.0	4.91	75.02	87.21	699.4	2.52	72.50	86.94
11.0	1300.0	4.82	77.33	87.45	816.3	2.94	74.39	87.15
12.0	1150.0	4.41	78.80	87.60	892.8	3.21	75.58	87.27
13.0	940.0	3.76	79.34	87.66	921.6	3.32	76.03	87.32
14.0	760.0	3.06	79.09	87.63	908.1	3.27	75.82	87.30
15.0	580.0	2.41	78.23	87.54	863.2	3.11	75.12	87.22
16.0	400.0	1.76	76.89	87.41	793.8	2.86	74.03	87.11
17.0	280.0	1.22	75.25	87.24	711.0	2.56	72.69	86.96
18.0	160.0	.79	73.48	87.05	623.7	2.25	71.24	86.81
19.0	100.0	.47	71.71	86.86	538.6	1.94	69.77	86.65
20.0	40.0	.25	70.02	86.67	460.6	1.66	68.36	86.49
21.0	.0	.07	68.43	86.50	390.0	1.40	67.03	86.34
22.0	.0	.00	67.03	86.34	330.1	1.19	65.84	86.21
23.0	.0	.00	65.84	86.21	281.4	1.01	64.83	86.09
24.0	.0	.00	64.83	86.09	241.6	.87	63.96	85.99
25.0	.0	.00	63.96	85.99	208.8	.75	63.21	85.90
26.0	.0	.00	63.21	85.90	181.6	.65	62.55	85.82
27.0	.0	.00	62.55	85.82	158.8	.57	61.98	85.76
28.0	.0	.00	61.98	85.76	139.6	.50	61.48	85.70
29.0	.0	.00	61.48	85.70	123.3	.44	61.04	85.64
30.0	.0	.00	61.04	85.64	109.4	.39	60.64	85.59

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 \* AMORTECIMENTO DA ONDA DE CHEIA \*  
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NIVEL DE PLENO ARMAZENAMENTO(m)..... 85.00  
 COTA DA SOLEIRA DESCARREGADORA(m)..... 85.00  
 COMPRIMENTO DA CRISTA(m).....120.00  
 COEFICIENTE DE VAZ|O..... .400

CURVA DE VOLUMES:V=a(Z-c)\*\*b  
 PARAMETRO a..... .0121610  
 PARAMETRO b..... 2.8147  
 PARAMETRO c..... 65.00

TEMPO (h)	Qaf1 (m3/s)	Vmed (hm3)	Vtot (hm3)	Zint (m)	Qdes m3/s)	Vdes (hm3)	Vfin (hm3)	Zfin (m)
.0	.0	.00	.00	85.00	.0	.00	55.84	.00
1.0	64.0	.12	55.96	85.01	.4	.00	55.95	85.01
2.0	232.0	.53	56.49	85.08	5.0	.02	56.47	85.08
3.0	464.0	1.25	57.72	85.24	24.5	.09	57.63	85.23
4.0	592.0	1.90	59.53	85.46	66.4	.24	59.30	85.43
5.0	1088.0	3.02	62.32	85.80	150.8	.54	61.78	85.73
6.0	1440.0	4.55	66.33	86.26	301.0	1.08	65.24	86.14
7.0	1760.0	5.76	71.00	86.78	505.7	1.82	69.18	86.58
8.0	2048.0	6.85	76.04	87.32	750.5	2.70	73.34	87.03
9.0	2160.0	7.57	80.91	87.82	1004.9	3.62	77.29	87.45
10.0	2208.0	7.86	85.15	88.23	1236.9	4.45	80.70	87.80
11.0	2080.0	7.72	88.42	88.55	1420.5	5.11	83.31	88.05
12.0	1840.0	7.06	90.36	88.73	1531.5	5.51	84.85	88.20
13.0	1504.0	6.02	90.87	88.78	1560.6	5.62	85.25	88.24
14.0	1216.0	4.90	90.15	88.71	1519.1	5.47	84.68	88.19
15.0	928.0	3.86	88.54	88.56	1427.1	5.14	83.40	88.06
16.0	640.0	2.82	86.22	88.34	1296.4	4.67	81.55	87.88
17.0	448.0	1.96	83.51	88.07	1146.1	4.13	79.39	87.66
18.0	256.0	1.27	80.65	87.79	991.2	3.57	77.09	87.43
19.0	160.0	.75	77.83	87.50	842.6	3.03	74.80	87.19
20.0	64.0	.40	75.20	87.23	708.6	2.55	72.65	86.96
21.0	.0	.12	72.77	86.97	589.1	2.12	70.65	86.74
22.0	.0	.00	70.65	86.74	489.2	1.76	68.89	86.55
23.0	.0	.00	68.89	86.55	409.8	1.48	67.41	86.38
24.0	.0	.00	67.41	86.38	346.1	1.25	66.16	86.24
25.0	.0	.00	66.16	86.24	294.4	1.06	65.10	86.12
26.0	.0	.00	65.10	86.12	252.3	.91	64.20	86.02
27.0	.0	.00	64.20	86.02	217.7	.78	63.41	85.92
28.0	.0	.00	63.41	85.92	188.9	.68	62.73	85.84
29.0	.0	.00	62.73	85.84	164.9	.59	62.14	85.77
30.0	.0	.00	62.14	85.77	144.8	.52	61.62	85.71

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 \* AMORTECIMENTO DA ONDA DE CHEIA \*  
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NIVEL DE PLENO ARMAZENAMENTO(m)..... 90.00  
 COTA DA SOLEIRA DESCARREGADORA(m)..... 90.00  
 COMPRIMENTO DA CRISTA(m)..... 60.00  
 COEFICIENTE DE VAZ||O..... .400

CURVA DE VOLUMES:V=a(Z-c)\*\*b  
 PARAMETRO a..... .0121610  
 PARAMETRO b..... 2.8147  
 PARAMETRO c..... 65.00

TEMPO (h)	Qafl (m3/s)	Vmed (hm3)	Vtot (hm3)	Zint (m)	Qdes (m3/s)	Vdes (hm3)	Vfin (hm3)	Zfin (m)
.0	.0	.00	.00	90.00	.0	.00	104.64	.00
1.0	40.0	.07	104.71	90.01	.0	.00	104.71	90.01
2.0	145.0	.33	105.04	90.03	.7	.00	105.04	90.03
3.0	290.0	.78	105.83	90.10	3.3	.01	105.81	90.10
4.0	370.0	1.19	107.00	90.20	9.4	.03	106.97	90.20
5.0	680.0	1.89	108.86	90.35	22.3	.08	108.78	90.35
6.0	900.0	2.84	111.62	90.58	46.9	.17	111.45	90.57
7.0	1100.0	3.60	115.05	90.86	84.2	.30	114.75	90.83
8.0	1280.0	4.28	119.03	91.17	134.7	.48	118.55	91.13
9.0	1350.0	4.73	123.28	91.50	195.1	.70	122.58	91.45
10.0	1380.0	4.91	127.49	91.82	260.4	.94	126.56	91.75
11.0	1300.0	4.82	131.38	92.10	324.6	1.17	130.21	92.02
12.0	1150.0	4.41	134.62	92.34	380.6	1.37	133.25	92.24
13.0	940.0	3.76	137.01	92.51	423.2	1.52	135.49	92.40
14.0	760.0	3.06	138.55	92.62	451.1	1.62	136.93	92.51
15.0	580.0	2.41	139.34	92.68	465.6	1.68	137.66	92.56
16.0	400.0	1.76	139.43	92.68	467.2	1.68	137.74	92.56
17.0	280.0	1.22	138.97	92.65	458.8	1.65	137.32	92.53
18.0	160.0	.79	138.11	92.59	443.1	1.60	136.51	92.48
19.0	100.0	.47	136.98	92.51	422.7	1.52	135.46	92.40
20.0	40.0	.25	135.71	92.42	399.9	1.44	134.27	92.32
21.0	.0	.07	134.34	92.32	375.7	1.35	132.99	92.22
22.0	.0	.00	132.99	92.22	352.2	1.27	131.72	92.13
23.0	.0	.00	131.72	92.13	330.4	1.19	130.53	92.04
24.0	.0	.00	130.53	92.04	310.3	1.12	129.42	91.96
25.0	.0	.00	129.42	91.96	291.7	1.05	128.37	91.88
26.0	.0	.00	128.37	91.88	274.5	.99	127.38	91.81
27.0	.0	.00	127.38	91.81	258.5	.93	126.45	91.74
28.0	.0	.00	126.45	91.74	243.7	.88	125.57	91.67
29.0	.0	.00	125.57	91.67	230.0	.83	124.74	91.61
30.0	.0	.00	124.74	91.61	217.2	.78	123.96	91.55



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 \* AMORTECIMENTO DA ONDA DE CHEIA \*  
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NIVEL DE PLENO ARMAZENAMENTO(m)..... 90.00  
 COTA DA SOLEIRA DESCARREGADORA(m)..... 90.00  
 COMPRIMENTO DA CRISTA(m)..... 60.00  
 COEFICIENTE DE VAZ||O..... .400

CURVA DE VOLUMES:V=a(Z-c)\*\*b  
 PARAMETRO a..... .0121610  
 PARAMETRO b..... 2.8147  
 PARAMETRO c..... 65.00

TEMPO (h)	Qafl (m3/s)	Vmed (hm3)	Vtot (hm3)	Zint (m)	Qdes (m3/s)	Vdes (hm3)	Vfin (hm3)	Zfin (m)
.0	.0	.00	.00	90.00	.0	.00	104.64	.00
1.0	64.0	.12	104.76	90.01	.1	.00	104.75	90.01
2.0	232.0	.53	105.29	90.05	1.3	.00	105.28	90.05
3.0	464.0	1.25	106.54	90.16	6.8	.02	106.51	90.16
4.0	592.0	1.90	108.41	90.32	18.9	.07	108.34	90.31
5.0	1088.0	3.02	111.37	90.56	44.4	.16	111.21	90.55
6.0	1440.0	4.55	115.76	90.91	92.7	.33	115.42	90.89
7.0	1760.0	5.76	121.18	91.34	164.5	.59	120.59	91.29
8.0	2048.0	6.85	127.45	91.81	259.6	.93	126.51	91.74
9.0	2160.0	7.57	134.09	92.30	371.2	1.34	132.75	92.20
10.0	2208.0	7.86	140.61	92.77	489.2	1.76	138.85	92.64
11.0	2080.0	7.72	146.57	93.18	602.5	2.17	144.40	93.03
12.0	1840.0	7.06	151.46	93.51	698.8	2.52	148.94	93.34
13.0	1504.0	6.02	154.96	93.74	769.4	2.77	152.19	93.56
14.0	1216.0	4.90	157.09	93.88	812.9	2.93	154.16	93.69
15.0	928.0	3.86	158.02	93.94	832.0	3.00	155.02	93.75
16.0	640.0	2.82	157.85	93.93	828.5	2.98	154.86	93.74
17.0	448.0	1.96	156.82	93.86	807.4	2.91	153.92	93.67
18.0	256.0	1.27	155.18	93.76	774.0	2.79	152.40	93.57
19.0	160.0	.75	153.15	93.62	732.7	2.64	150.51	93.45
20.0	64.0	.40	150.91	93.47	687.9	2.48	148.43	93.31
21.0	.0	.12	148.55	93.31	641.2	2.31	146.24	93.16
22.0	.0	.00	146.24	93.16	596.1	2.15	144.09	93.01
23.0	.0	.00	144.09	93.01	554.9	2.00	142.10	92.87
24.0	.0	.00	142.10	92.87	517.0	1.86	140.24	92.74
25.0	.0	.00	140.24	92.74	482.2	1.74	138.50	92.62
26.0	.0	.00	138.50	92.62	450.2	1.62	136.88	92.50
27.0	.0	.00	136.88	92.50	420.8	1.51	135.36	92.39
28.0	.0	.00	135.36	92.39	393.7	1.42	133.95	92.29
29.0	.0	.00	133.95	92.29	368.8	1.33	132.62	92.20
30.0	.0	.00	132.62	92.20	345.8	1.24	131.37	92.10

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 \* AMORTECIMENTO DA ONDA DE CHEIA \*  
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NIVEL DE PLENO ARMAZENAMENTO(m)..... 95.00  
 COTA DA SOLEIRA DESCARREGADORA(m)..... 95.00  
 COMPRIMENTO DA CRISTA(m)..... 20.00  
 COEFICIENTE DE VAZ||O..... .400

CURVA DE VOLUMES:V=a(Z-c)\*\*b  
 PARAMETRO a..... .0121610  
 PARAMETRO b..... 2.8147  
 PARAMETRO c..... 65.00

TEMPO (h)	Qaf1 (m3/s)	Vmed (hm3)	Vtot (hm3)	Zint (m)	Qdes (m3/s)	Vdes (hm3)	Vfin (hm3)	Zfin (m)
.0	.0	.00	.00	95.00	.0	.00	174.82	.00
1.0	40.0	.07	174.89	95.00	.0	.00	174.89	95.00
2.0	145.0	.33	175.22	95.02	.1	.00	175.22	95.02
3.0	290.0	.78	176.01	95.07	.7	.00	176.01	95.07
4.0	370.0	1.19	177.19	95.14	1.9	.01	177.19	95.14
5.0	680.0	1.89	179.08	95.26	4.6	.02	179.06	95.26
6.0	900.0	2.84	181.90	95.43	9.9	.04	181.87	95.42
7.0	1100.0	3.60	185.47	95.64	18.0	.06	185.40	95.63
8.0	1280.0	4.28	189.69	95.88	29.4	.11	189.58	95.88
9.0	1350.0	4.73	194.32	96.15	43.6	.16	194.16	96.14
10.0	1380.0	4.91	199.07	96.42	59.8	.22	198.86	96.40
11.0	1300.0	4.82	203.68	96.67	76.7	.28	203.41	96.66
12.0	1150.0	4.41	207.82	96.90	92.8	.33	207.48	96.88
13.0	940.0	3.76	211.24	97.09	106.8	.38	210.86	97.07
14.0	760.0	3.06	213.92	97.23	118.0	.42	213.49	97.21
15.0	580.0	2.41	215.91	97.34	126.5	.46	215.45	97.31
16.0	400.0	1.76	217.21	97.41	132.2	.48	216.74	97.38
17.0	280.0	1.22	217.96	97.45	135.5	.49	217.47	97.42
18.0	160.0	.79	218.27	97.46	136.8	.49	217.77	97.44
19.0	100.0	.47	218.24	97.46	136.7	.49	217.75	97.43
20.0	40.0	.25	218.00	97.45	135.7	.49	217.51	97.42
21.0	.0	.07	217.58	97.43	133.8	.48	217.10	97.40
22.0	.0	.00	217.10	97.40	131.7	.47	216.63	97.37
23.0	.0	.00	216.63	97.37	129.7	.47	216.16	97.35
24.0	.0	.00	216.16	97.35	127.6	.46	215.70	97.33
25.0	.0	.00	215.70	97.33	125.7	.45	215.25	97.30
26.0	.0	.00	215.25	97.30	123.7	.45	214.80	97.28
27.0	.0	.00	214.80	97.28	121.8	.44	214.37	97.25
28.0	.0	.00	214.37	97.25	119.9	.43	213.93	97.23
29.0	.0	.00	213.93	97.23	118.1	.43	213.51	97.21
30.0	.0	.00	213.51	97.21	116.3	.42	213.09	97.19

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 \* AMORTECIMENTO DA ONDA DE CHEIA \*  
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NIVEL DE PLENO ARMAZENAMENTO(m)..... 95.00  
 COTA DA SOLEIRA DESCARREGADORA(m)..... 95.00  
 COMPRIMENTO DA CRISTA(m)..... 20.00  
 COEFICIENTE DE VAZ||O..... .400

CURVA DE VOLUMES:V=a(Z-c)\*\*b  
 PARAMETRO a..... .0121610  
 PARAMETRO b..... 2.8147  
 PARAMETRO c..... 65.00

TEMPO (h)	Qafl (m3/s)	Vmed (hm3)	Vtot (hm3)	Zint (m)	Qdes (m3/s)	Vdes (hm3)	Vfin (hm3)	Zfin (m)
.0	.0	.00	.00	95.00	.0	.00	174.82	.00
1.0	64.0	.12	174.94	95.01	.0	.00	174.94	95.01
2.0	232.0	.53	175.47	95.04	.3	.00	175.47	95.04
3.0	464.0	1.25	176.72	95.12	1.4	.00	176.71	95.11
4.0	592.0	1.90	178.62	95.23	3.9	.01	178.60	95.23
5.0	1088.0	3.02	181.63	95.41	9.3	.03	181.59	95.41
6.0	1440.0	4.55	186.14	95.68	19.7	.07	186.07	95.67
7.0	1760.0	5.76	191.83	96.01	35.8	.13	191.70	96.00
8.0	2048.0	6.85	198.56	96.39	58.0	.21	198.35	96.38
9.0	2160.0	7.57	205.92	96.80	85.4	.31	205.62	96.78
10.0	2208.0	7.86	213.48	97.21	116.1	.42	213.06	97.18
11.0	2080.0	7.72	220.78	97.59	148.0	.53	220.25	97.57
12.0	1840.0	7.06	227.30	97.93	178.0	.64	226.66	97.90
13.0	1504.0	6.02	232.68	98.21	203.5	.73	231.95	98.17
14.0	1216.0	4.90	236.84	98.42	223.8	.81	236.04	98.38
15.0	928.0	3.86	239.90	98.57	239.0	.86	239.04	98.53
16.0	640.0	2.82	241.86	98.67	248.8	.90	240.96	98.62
17.0	448.0	1.96	242.92	98.72	254.2	.92	242.01	98.67
18.0	256.0	1.27	243.27	98.74	256.0	.92	242.35	98.69
19.0	160.0	.75	243.10	98.73	255.1	.92	242.18	98.68
20.0	64.0	.40	242.59	98.70	252.5	.91	241.68	98.66
21.0	.0	.12	241.79	98.66	248.5	.89	240.90	98.62
22.0	.0	.00	240.90	98.62	244.0	.88	240.02	98.58
23.0	.0	.00	240.02	98.58	239.6	.86	239.16	98.53
24.0	.0	.00	239.16	98.53	235.3	.85	238.31	98.49
25.0	.0	.00	238.31	98.49	231.1	.83	237.48	98.45
26.0	.0	.00	237.48	98.45	227.0	.82	236.66	98.41
27.0	.0	.00	236.66	98.41	222.9	.80	235.86	98.37
28.0	.0	.00	235.86	98.37	219.0	.79	235.07	98.33
29.0	.0	.00	235.07	98.33	215.1	.77	234.29	98.29
30.0	.0	.00	234.29	98.29	211.4	.76	233.53	98.25

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 \*  
 \* AMORTECIMENTO DA ONDA DE CHEIA \*  
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NIVEL DE PLENO ARMAZENAMENTO(m)..... 95.00  
 COTA DA SOLEIRA DESCARREGADORA(m)..... 95.00  
 COMPRIMENTO DA CRISTA(m)..... 20.00  
 COEFICIENTE DE VAZ||O..... .400

CURVA DE VOLUMES:V=a(Z-c)\*\*b  
 PARAMETRO a..... .0121610  
 PARAMETRO b..... 2.8147  
 PARAMETRO c..... 65.00

TEMPO (h)	Qaf1 (m3/s)	Vmed (hm3)	Vtot (hm3)	Zint (m)	Qdes (m3/s)	Vdes (hm3)	Vfin (hm3)	Zfin (m)
.0	.0	.00	.00	95.00	.0	.00	174.82	.00
1.0	40.0	.07	174.89	95.00	.0	.00	174.89	95.00
2.0	145.0	.33	175.22	95.02	.1	.00	175.22	95.02
3.0	290.0	.78	176.01	95.07	.7	.00	176.01	95.07
4.0	370.0	1.19	177.19	95.14	1.9	.01	177.19	95.14
5.0	680.0	1.89	179.08	95.26	4.6	.02	179.06	95.26
6.0	900.0	2.84	181.90	95.43	9.9	.04	181.87	95.42
7.0	1100.0	3.60	185.47	95.64	18.0	.06	185.40	95.63
8.0	1280.0	4.28	189.69	95.88	29.4	.11	189.58	95.88
9.0	1350.0	4.73	194.32	96.15	43.6	.16	194.16	96.14
10.0	1380.0	4.91	199.07	96.42	59.8	.22	198.86	96.40
11.0	1300.0	4.82	203.68	96.67	76.7	.28	203.41	96.66
12.0	1150.0	4.41	207.82	96.90	92.8	.33	207.48	96.88
13.0	940.0	3.76	211.24	97.09	106.8	.38	210.86	97.07
14.0	760.0	3.06	213.92	97.23	118.0	.42	213.49	97.21
15.0	580.0	2.41	215.91	97.34	126.5	.46	215.45	97.31
16.0	400.0	1.76	217.21	97.41	132.2	.48	216.74	97.38
17.0	280.0	1.22	217.96	97.45	135.5	.49	217.47	97.42
18.0	160.0	.79	218.27	97.46	136.8	.49	217.77	97.44
19.0	100.0	.47	218.24	97.46	136.7	.49	217.75	97.43
20.0	40.0	.25	218.00	97.45	135.7	.49	217.51	97.42
21.0	.0	.07	217.58	97.43	133.8	.48	217.10	97.40
22.0	.0	.00	217.10	97.40	131.7	.47	216.63	97.37
23.0	.0	.00	216.63	97.37	129.7	.47	216.16	97.35
24.0	.0	.00	216.16	97.35	127.6	.46	215.70	97.33
25.0	.0	.00	215.70	97.33	125.7	.45	215.25	97.30
26.0	.0	.00	215.25	97.30	123.7	.45	214.80	97.28
27.0	.0	.00	214.80	97.28	121.8	.44	214.37	97.25
28.0	.0	.00	214.37	97.25	119.9	.43	213.93	97.23
29.0	.0	.00	213.93	97.23	118.1	.43	213.51	97.21
30.0	.0	.00	213.51	97.21	116.3	.42	213.09	97.19

\*\*\*\*\*  
 \*  
 \* AMORTECIMENTO DA ONDA DE CHEIA \*  
 \*\*\*\*\*

NIVEL DE PLENO ARMAZENAMENTO(m)..... 95.00  
 COTA DA SOLEIRA DESCARREGADORA(m)..... 95.00  
 COMPRIMENTO DA CRISTA(m)..... 20.00  
 COEFICIENTE DE VAZ||O..... .400

CURVA DE VOLUMES:V=a(Z-c)\*\*b  
 PARAMETRO a..... .0121610  
 PARAMETRO b..... 2.8147  
 PARAMETRO c..... 65.00

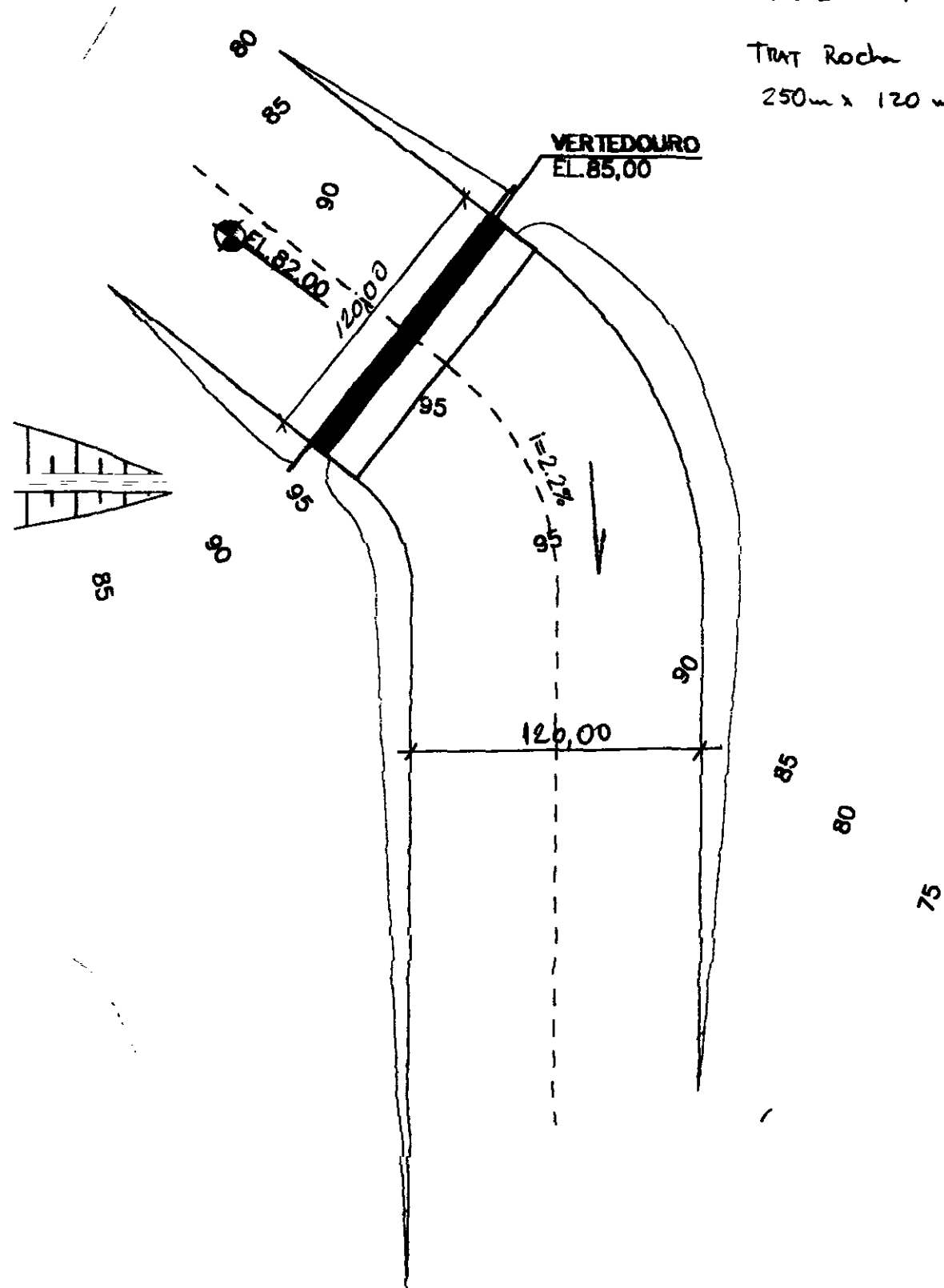
TEMPO (h)	Qaf1 (m3/s)	Vmed (hm3)	Vtot (hm3)	Zint (m)	Qdes (m3/s)	Vdes (hm3)	Vfin (hm3)	Zfin (m)
.0	.0	.00	.00	95.00	.0	.00	174.82	.00
1.0	64.0	.12	174.94	95.01	.0	.00	174.94	95.01
2.0	232.0	.53	175.47	95.04	.3	.00	175.47	95.04
3.0	464.0	1.25	176.72	95.12	1.4	.00	176.71	95.11
4.0	592.0	1.90	178.62	95.23	3.9	.01	178.60	95.23
5.0	1088.0	3.02	181.63	95.41	9.3	.03	181.59	95.41
6.0	1440.0	4.55	186.14	95.68	19.7	.07	186.07	95.67
7.0	1760.0	5.76	191.83	96.01	35.8	.13	191.70	96.00
8.0	2048.0	6.85	198.56	96.39	58.0	.21	198.35	96.38
9.0	2160.0	7.57	205.92	96.80	85.4	.31	205.62	96.78
10.0	2208.0	7.86	213.48	97.21	116.1	.42	213.06	97.18
11.0	2080.0	7.72	220.78	97.59	148.0	.53	220.25	97.57
12.0	1840.0	7.06	227.30	97.93	178.0	.64	226.66	97.90
13.0	1504.0	6.02	232.68	98.21	203.5	.73	231.95	98.17
14.0	1216.0	4.90	236.84	98.42	223.8	.81	236.04	98.38
15.0	928.0	3.86	239.90	98.57	239.0	.86	239.04	98.53
16.0	640.0	2.82	241.86	98.67	248.8	.90	240.96	98.62
17.0	448.0	1.96	242.92	98.72	254.2	.92	242.01	98.67
18.0	256.0	1.27	243.27	98.74	256.0	.92	242.35	98.69
19.0	160.0	.75	243.10	98.73	255.1	.92	242.18	98.68
20.0	64.0	.40	242.59	98.70	252.5	.91	241.68	98.66
21.0	.0	.12	241.79	98.66	248.5	.89	240.90	98.62
22.0	.0	.00	240.90	98.62	244.0	.88	240.02	98.58
23.0	.0	.00	240.02	98.58	239.6	.86	239.16	98.53
24.0	.0	.00	239.16	98.53	235.3	.85	238.31	98.49
25.0	.0	.00	238.31	98.49	231.1	.83	237.48	98.45
26.0	.0	.00	237.48	98.45	227.0	.82	236.66	98.41
27.0	.0	.00	236.66	98.41	222.9	.80	235.86	98.37
28.0	.0	.00	235.86	98.37	219.0	.79	235.07	98.33
29.0	.0	.00	235.07	98.33	215.1	.77	234.29	98.29
30.0	.0	.00	234.29	98.29	211.4	.76	233.53	98.25

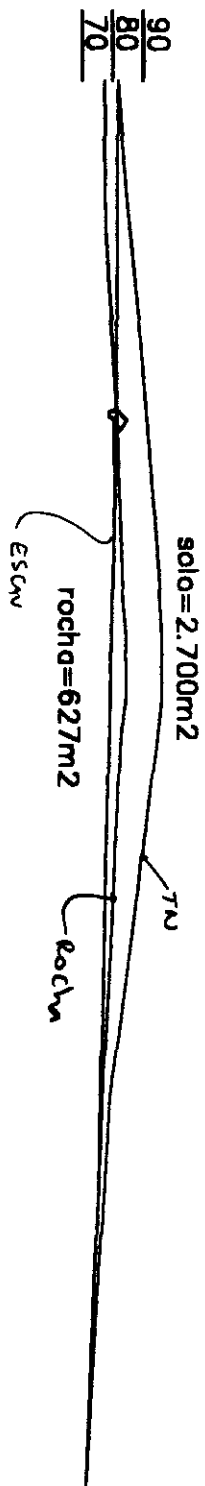
BARRAGEM ARACOIABA  
CRISTA EL. 90,00  
N.A. EL. 85,00

DESMA TAMENTO  
400 m x 130 m = 52 000 m<sup>2</sup>

TRAT Solo  
110 m x 120 m = 13 200 m<sup>2</sup>

TRAT Rocha  
250 m x 120 m = 30 000 m<sup>2</sup>





GALERIA DE DESVIO 28% MENOR QUE ALT. 1

Escav solo - 2 030

Rocha - 510

TRAT. solo - 360

Rocha - 830

CONC c/FORMA - ~~900~~ 1.270 m<sup>3</sup>

s/FORMA - ~~420~~ 310 m<sup>3</sup>

CRISTA EL 90,00

## 1- ESCAVAÇÃO

Solo -  $l$  média: 130,0 m

$$\text{Volume} = 2700 \text{ m}^2 \times 130,0 = 351.000 \text{ m}^3$$

Rocha  $l$  média = 123,0 m

$$\text{Volume} = 627,0 \text{ m}^2 \times 123,0 \text{ m} = 77.120 \text{ m}^3$$

## 2- CONCRETO

largura do vert = 120,00 m

CONCRETO S/FORMA

$$\text{Se } \cdot \text{ p/ } 60,0 \text{ m} = 600,00 \text{ m}^3$$

$$\text{p/ } 120,0 \text{ m} = \text{X}$$

$$\text{X} = \underline{1.200 \text{ m}^3}$$

CONCRETO G/FORMA

$$\text{Se } \cdot \text{ p/ } 60,0 \text{ m} = 1510,0 \text{ m}^3$$

$$\text{p/ } 120,0 \text{ m} = \text{X}$$

$$\text{X} = 3020 \text{ m}^3$$

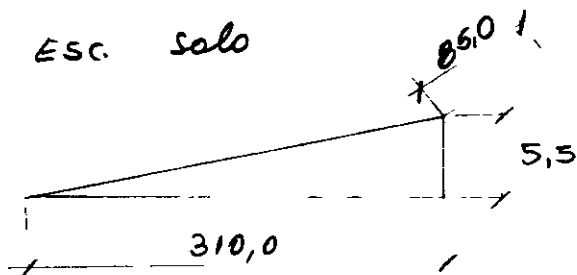


CRISTA EL. 95,00  
N.A EL. 90,00

## 1- ESCAVAÇÃO

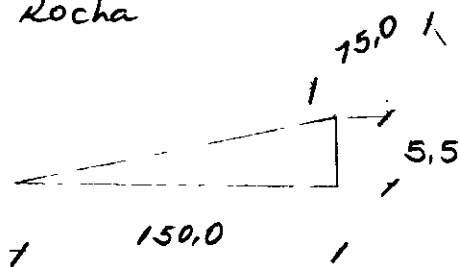
## 1.1. CANAL DE APROXIMAÇÃO

ESC. Solo



$$\text{Vol} = \frac{310,0 \times 5,5}{2} \times 85,0 = 72.460 \text{ m}^3$$

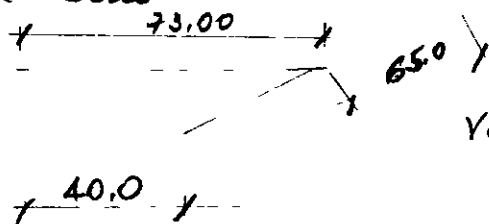
ESC. Rocha



$$\text{Vol} = \frac{150,0 \times 5,5}{2} \times 15,0 = 30.930 \text{ m}^3$$

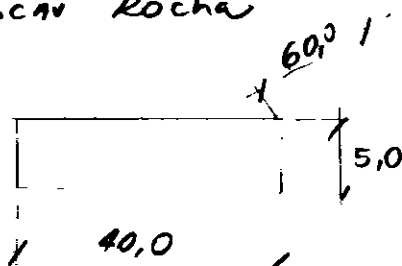
## 1.2. VERTEDEIRO

ESC. Solo

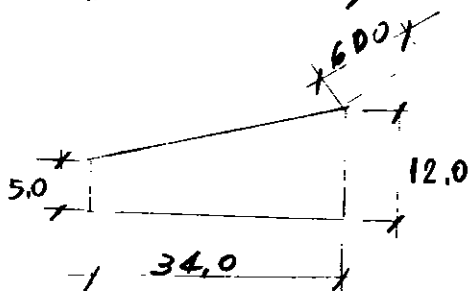


$$\text{Vol} = \frac{73,0 + 40,0}{2} \times 65,0 = 3.670 \text{ m}^3$$

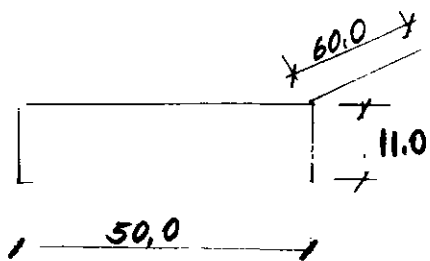
ESCAV. Rocha



$$\text{Vol} = \frac{40,0 \times 5,0}{2} \times 60,0 = 12.000 \text{ m}^3$$



$$\text{Vol} = \left[ \frac{(5,0 + 12,0) \times 34,0}{2} \right] \times 60,0 = 17.340 \text{ m}^3$$

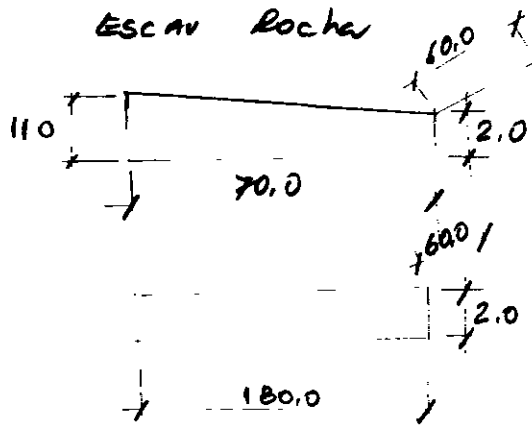


$$\text{Vol} = 50,0 \times 11,0 \times 60,0 = 33.000 \text{ m}^3$$

### 1.3 - CANAL DE RESTITUIÇÃO

ESCAVA. SOLO

$$200,0 \times 2,5 \times 65,0 = 32.500,00 \text{ m}^3$$



$$\text{Vol} = \left( \frac{11,0 + 180,0}{2} \times 70,0 \right) \times 60,0 = 27.300 \text{ m}^3$$

$$\text{Vol} = 180,0 \times 2,0 \times 60,0 = 21.600 \text{ m}^3$$

RESUMO ESCAVAÇÃO

Solo - 108.630 m<sup>3</sup>

Rocha - 142.200 m<sup>3</sup>

## 2 - CONCRETO

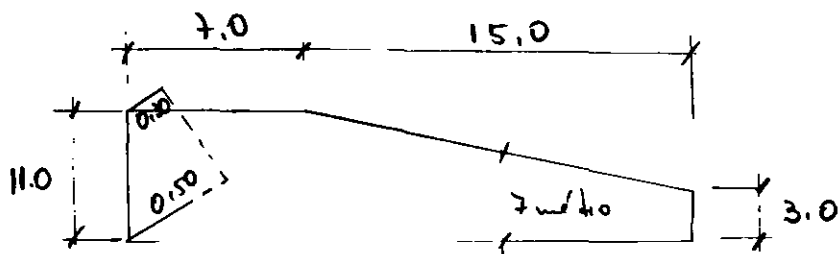
### 2.1 - VERTEDOURO

$$\left( \frac{2,50 + 1,50}{2} \times 2,0 \right) \times 60,0 = 240,0 \text{ m}^3$$

$$\left( \frac{4,0 + 1,0}{2} \right) \times 3,0 \times 60,0 = 450,0 \text{ m}^3$$

$$\left( \frac{4,0 + 0,5}{2} \times 5,0 \right) \times 60,0 = \frac{675,0 \text{ m}^3}{1.365,0 \text{ m}^3}$$

### 2.2 MURROS VERTEDOURO



$$\text{Vol} = \frac{(0.30 + 0.50)}{2} \times 11.0 \times 7.0 = 30.80 \text{ m}^3$$

$$\text{Vol} = \frac{(0.30 + 0.50)}{2} \times 7.0 \times 15.0 = 42.00 \text{ m}^3$$

$$72.80 \times 2 = 145.60 \text{ m}^3$$

2 3 LAJE  
esp = 0.50

$$\text{Vol} = 20.0 \times 60.0 \times 0.50 = 600.0 \text{ m}^3$$

### RESUMO

CONCRETO S/ FORMA - 600,0 m<sup>3</sup>  
C/ FORMA - 1.510,0 m<sup>3</sup>

TRATAMENTO DA ESC. SOLO

$$160,0 \times 70,0 = 11200 \text{ m}^2$$

TRATAMENTO DA ESC ROCHA

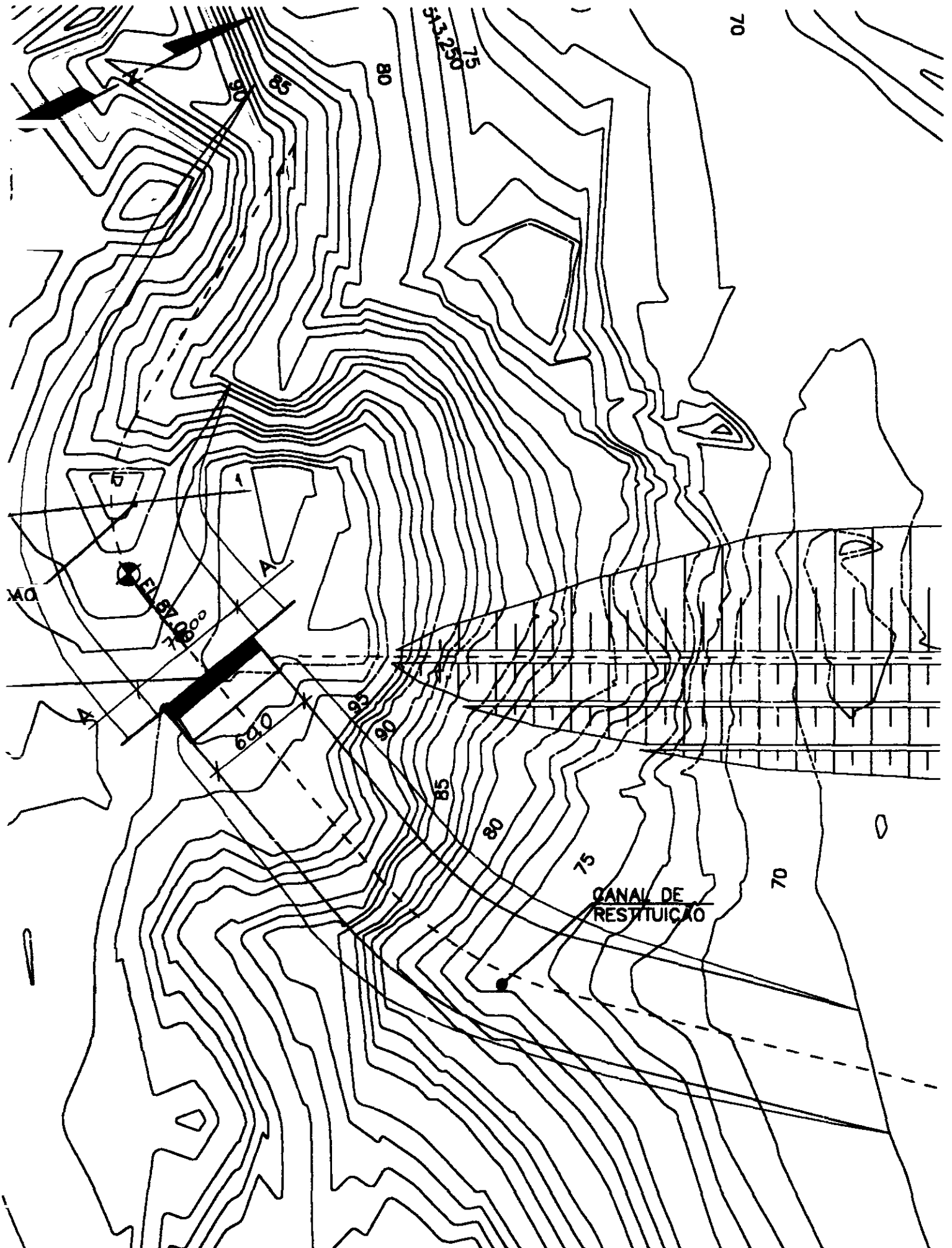
$$600,0 \text{ m} \times 60,0 = 36000 \text{ m}^2$$

DESMATAMENTO

$$810,0 \text{ m} \times 70,0 = 56700 \text{ m}^2$$

GALERIA DE DESVIO - 15% MENOR QUE ALTERNATIVA 1  
 ESCAV SOLO 2300 m<sup>3</sup> TRATA SOLO 400 m<sup>2</sup>  
 Rocha 600 m<sup>2</sup> Rocha 940 m<sup>2</sup>

CONCRETO C/ FORMA 1440 m<sup>3</sup>  
S/ FORMA 350 m<sup>3</sup>



**ANEXO 6.2  
OBRAS DE TERRA**

Tal Mont (1V xH) 2,2  
 Tal Jus (1V xH) 2  
 Largura Crista(m) 7  
 Cota Crista(m) 100  
 Altura Bermas(m) 10  
 Largura Bermas(m) 3  
 Espessura Escav (m) 1

**BARRAGEM DE ARACOIABA - CE**  
**ALTERNATIVA 1 - NA 86,00**  
 Estudos de Otimização  
 Cálculo de Volumes e Áreas  
 Terreno Natural

Bermas	G
0	0
1	0
2	1
3	3
4	6

Ponto	Dist	Terr Nat	Terr. Escav	Altura	Bermas	Var=(G)	Base	Area Transv	Volume	Dist	Area Fund
Estudo	(x)	(y)	(V)	(H)	(G)	(J)	(L)	(R)	(V)	(P)	(S)
1	0,00	100,00	99,00	1,00	0	0	11,20	9,10	0,00	0,00	0,00
2	120,00	99,00	98,00	2,00	0	0	15,40	22,40	1 890,00	120,00	1 596,08
3	98,00	98,00	97,00	3,00	0	0	19,80	39,90	2 055,90	98,01	1 155,13
4	180,00	98,00	97,00	3,00	0	0	19,80	39,90	7 182,00	180,00	3 528,00
5	50,00	97,00	96,00	4,00	0	0	23,80	61,80	2 537,50	50,01	1 085,22
6	20,00	100,00	97,00	3,00	0	0	19,80	39,90	1 015,00	20,02	434,54
7	15,00	98,00	97,00	3,00	0	0	19,80	39,90	596,50	15,00	294,00
8	86,00	98,00	97,00	3,00	0	0	19,80	39,90	3 431,40	86,00	1 685,80
9	15,00	96,00	94,00	6,00	0	0	32,20	117,60	1 181,25	15,30	398,19
10	20,00	90,00	89,00	11,00	1	0	58,20	334,10	4 517,00	20,82	911,21
11	30,00	85,00	84,00	16,00	1	0	77,20	667,60	15 025,50	30,41	2 028,60
12	58,00	80,00	79,00	21,00	2	1	101,20	1 109,10	51 524,30	58,22	5 182,78
13	48,00	75,00	74,00	26,00	2	1	122,20	1 667,60	68 840,80	48,26	5 390,61
14	35,00	70,00	69,00	31,00	3	3	146,20	2 334,10	70 029,75	35,36	4 744,69
15	55,00	68,00	67,00	33,00	3	3	154,60	2 634,90	136 647,50	55,04	8 277,47
16	120,00	69,00	68,00	32,00	3	3	150,40	2 482,40	307 038,00	120,00	18 300,84
17	40,00	66,00	65,00	35,00	3	3	163,00	2 952,50	108 698,00	40,11	6 285,80
18	10,00	68,00	67,00	33,00	3	3	154,60	2 634,90	27 937,00	10,20	1 819,45
19	30,00	69,00	68,00	32,00	3	3	150,40	2 482,40	78 759,50	30,02	4 577,54
20	30,00	70,00	69,00	31,00	3	3	146,20	2 334,10	72 247,50	30,02	4 451,47
21	85,00	75,00	74,00	26,00	2	3	122,20	1 727,60	172 622,25	85,15	11 428,72
22	70,00	80,00	79,00	21,00	2	1	101,20	1 109,10	99 284,50	70,18	7 838,92
23	50,00	86,00	84,00	16,00	1	0	77,20	667,60	44 417,50	50,25	4 482,24
24	35,00	80,00	80,00	11,00	1	0	58,20	334,10	17 529,75	35,36	2 358,20
25	50,00	96,00	94,00	6,00	0	0	32,20	117,60	11 262,50	50,25	2 221,02
26	54,00	97,00	96,00	4,00	0	0	23,80	61,80	4 836,40	54,04	1 513,04
27	68,00	95,00	94,00	6,00	0	0	32,20	117,60	6 082,80	68,03	1 904,82
28	90,00	91,00	90,00	10,00	0	0	49,00	280,00	17 962,00	90,09	3 657,61
29	125,00	91,00	90,00	10,00	0	0	49,00	280,00	35 000,00	125,00	6 125,00
30	90,00	83,00	82,00	8,00	0	0	40,80	190,40	21 168,00	90,02	4 033,00
31	100,00	96,00	94,00	6,00	0	0	32,20	117,60	15 400,00	100,02	3 640,73
32	65,00	97,00	96,00	4,00	0	0	23,80	61,80	5 824,00	65,03	1 820,86
33	70,00	100,00	99,00	1,00	0	0	11,20	9,10	2 474,50	70,06	1 226,12

1.980,00  
 COMPR DO  
 EIXO

1 410 792,60  
 VOLUME DE  
 MACIÇO

124 203,08  
 ÁREA DE  
 FUNDAÇÃO

# ESTUDO DE OTIMIZAÇÃO ALTERNATIVA 1

COTA DE CRISTA - 100,00

COTA DO NAmx - 95,00

Volume Total - 1482 300

- (S) 1 311 000 - 88,4 %
- (F) 86 100 - 5,5 %
- (T) 25 900 - 1,8 %
- (RR) 25 700 - 1,8 %
- (E) 31 600 - 2,2 %

Volume Total Compensado - 1410 600 m<sup>3</sup>

- (S) 1 247 200 m<sup>3</sup>
- (F) 81 800 m<sup>3</sup>
- (T) 25 400 m<sup>3</sup>
- (RR) 25 400 m<sup>3</sup>
- (E) 31 000 m<sup>3</sup>

Desmatamento - 125 000 - 8% = 115 000 m<sup>2</sup>

Tratamento em solo - 85 000 - 8% = 78 200 m<sup>2</sup>

Tratamento em rocha - 40 000 - 8% = 36 800 m<sup>2</sup>

## Escavações

Comum - 183 200 - 8% = 168 600 m<sup>3</sup>

Rocha - 10 000 m<sup>3</sup>

## Ensecadeiras

(S) 157 000 m<sup>3</sup>

(E) 0 000 m<sup>3</sup>

Miscão - 51 000 m<sup>3</sup>

Tal Mont (1V xH) 2,2  
 Tal Jus (1V xH) 2  
 Largura Crista(m) 7  
 Cota Crista(m) 95  
 Altura Barmas(m) 10  
 Largura Barmas(m) 3  
 Espessura Escav (m) 1

**BARRAGEM DE ARACOIABA - CE**  
**ALTERNATIVA 2 - NA 90,00**  
 Estudos de Otimização  
 Cálculo de Volumes e Áreas FI 1/2  
 Terreno Natural

Barmas	G
0	0
1	0
2	1
3	3
4	6

Ponto	Dist	Terr Nat	Terr Escav.	Altura	Barmas	Vaz=(G)	Base	Area Transv	Volume	Dist	Area Fund
Estudo	(x)	(y)	(y')	(H)	(G)	(J)	(L)	(R)	(V)	(P)	(S)
1	0,00	95,00	94,00	1,00	0	0	11,20	9,10	0,00	0,00	0,00
2	50,00	85,00	84,00	11,00	1	0	56,20	334,10	8.580,00	50,89	1 718,37
3	50,00	80,50	78,80	15,50	1	0	75,10	626,53	24.080,63	50,20	3 285,77
4	50,00	78,00	75,00	20,00	1	0	94,00	1 010,00	40.988,13	50,20	4.244,59
5	40,00	70,00	68,00	26,00	2	0	122,20	1.637,60	52.962,00	40,46	4.372,37
6	40,00	69,00	66,00	27,00	2	0	126,40	1 761,90	67.960,00	40,01	4 973,55
7	146,00	69,00	66,00	27,00	2	0	126,40	1 761,90	258.475,50	146,00	18 328,00
8	30,00	68,00	65,00	30,00	2	0	136,00	2 160,00	58.826,50	30,15	4.000,86
9	56,00	68,00	65,00	27,00	2	0	126,40	1 761,90	107 852,25	56,08	7 309,35
10	20,00	70,00	69,00	26,00	2	0	122,20	1 637,60	33.965,00	20,02	2 489,11
11	50,00	73,00	72,00	23,00	2	0	106,60	1 286,90	73 167,50	50,09	5 805,42
12	100,00	60,00	79,00	16,00	1	1	77,20	667,60	69 375,00	100,24	9 362,86
13	85,00	80,00	89,00	6,00	0	1	32,20	147,60	35.621,00	85,59	4 661,57
14	50,00	95,00	94,00	1,00	0	3	11,20	99,10	6.167,50	50,25	1 060,41

785,00  
 COMPR DO  
 EIXO

865.403,00  
 VOLUME DE  
 MACIÇO

71 672,22  
 ÁREA DE  
 FUNDAÇÃO



Tal Mont (1V.xH) 2,2  
 Tal Jus.(1V.xH) 2  
 Largura Crista(m) 7  
 Cota Crista(m) 95  
 Altura Barmas(m) 10  
 Largura Barmas(m) 3  
 Espessura Escav (m) 0,5

**BARRAGEM DE ARACOIABA - CE**  
**ALTERNATIVA 2 - NA 90,00**  
 Estudos de Otimização  
 Cálculo de Volumes e Áreas FI 2/2  
 Terreno Natural

Bermas	G
0	0
1	0
2	1
3	3
4	6

Ponto	Dist	Terr Nat	Terr Escav	Altura	Bermas	Var=4(G)	Base	Area Transv	Volume	Dist	Area Fund
Estude	(X)	(Y)	(Y')	(H)	(G)	(J)	(L)	(R)	(V)	(P)	(S)
1	0,00	95,00	94,50	0,50	0	0	9,10	4,03	0,00	0,00	0,00
2	90,00	72,00	71,50	23,50	2	0	111,70	1 345,23	60 716,25	92,89	5 610,70
3	100,00	90,50	90,00	5,00	0	0	28,00	87,50	71 636,25	101,70	7 103,53
4	95,00	92,00	91,50	3,50	0	0	21,70	50,23	5 853,31	85,01	2 112,58
5	100,00	94,00	93,50	1,50	0	0	13,30	15,23	3 272,50	100,02	1 750,35
6	90,00	95,00	94,50	0,50	0	0	9,10	4,03	481,25	50,01	560,11

425,00  
 COMPR DO  
 EIXO

141 959,56  
 VOLUME DE  
 MACIÇO

17 137,27  
 ÁREA DE  
 FUNDAÇÃO

Dreno de 26'

⊙  $(0,5 \times 5) \times 700 = 1 750$   
 ⊙  $20m^2 \times 700 = 14 000$  /  $5 = 50m^3$

$V_{COMPARTIM.} = 865 400 + 4 960 + 15 750 = 1 023 110 m^3$

# ESTUDO DE OTIMIZAÇÃO

## ALTERNATIVA 2

COTA DA CRISTA - 95,00

COTA DO NAMAX - 90,00

Volume Total Compensado

$$V = 1.023.110 \text{ m}^3 \quad (-48\% \text{ de alteração})$$

- Ⓒ 88,4%
- Ⓕ 5,8%
- Ⓙ 1,8%
- Ⓝ 1,5%
- Ⓔ 2,2%

Volumes Totais

- Ⓒ 904.500 m<sup>3</sup>
- Ⓕ 59.400 m<sup>3</sup>
- Ⓙ 18.500 m<sup>3</sup>
- ⒸⒸ 18.500 m<sup>3</sup>
- Ⓕ 22.500 m<sup>3</sup>

Desmatamento - 5000 - 28% = 82.800 m<sup>2</sup>

Tratamento em solo - 78200 - 28% = 56.300 m<sup>2</sup>

Tratamento em rocha - 36600 - 28% = 26.500 m<sup>2</sup>

Escavação

Comum - 168.600 - 28% = 121.400 m<sup>3</sup>

Rocha - 8.000 m<sup>3</sup>

Ensecad. RAS (xlt 2)

L - Ens 1ª FASC ( $l = 40 - 20 = 390$ )

$$MD - (84/2 \times 180) + (84 \times 390) = 40.320$$

$$ME - (84/2 \times 210) + (84 \times 390) = 53.180$$

} 73.500

2 - Ens. 2ª FASC

$$LUS - 9.800 \text{ m}^3$$

$$MON - 55.320 \text{ m}^3$$

$$\text{Total} = \textcircled{3} 138.600 \text{ m}^3 + 14.900 \text{ m}^3 = 153.500 \text{ m}^3$$

CONCLUSIÓN

$$MD - 84 \times 300 = 25.200$$

$$ME - (84 \times 300) + 5.355 = 30.555$$

$$\text{Total} = 47.400 \text{ m}^3$$

Tal Mort (1V:3d) 2,2  
 Tal Jus (1V:3d) 2  
 Largura Crista(m) 7  
 Cota Crista(m) 90  
 Altura Bermas(m) 10  
 Largura Bermas(m) 3  
 Espessura Escav (m) 1

**BARRAGEM DE ARACOIABA - CE**  
**ALTERNATIVA 3 - NA 85,00**  
 Estudos de Otimização  
 Cálculo de Volumes e Áreas  
 Terreno Natural

Bermas	G
0	0
1	0
2	1
3	3
4	6

Ponto	Dist	Terr Nat	Terr Escav	Altura	Bermas	Var=f(G)	Bese	Area Transv	Volume	Dist	Area Fund
Estudo	(x)	(y)	(y')	(H)	(G)	(J)	(L)	(R)	(V)	(F)	(S)
1	0,00	90,00	89,00	1,00	0	0	11,20	9,10	0,00	0,00	0,00
2	30,00	85,00	84,00	8,00	0	0	32,20	117,80	1 900,50	30,41	858,98
3	55,00	80,00	79,00	11,00	1	0	56,20	334,10	12 421,75	55,23	2 441,02
4	50,00	75,00	74,00	16,00	1	0	77,20	667,80	25 042,50	50,25	3 361,83
5	35,00	70,00	69,00	21,00	2	0	101,20	1 079,10	30 567,25	35,36	3 153,70
6	40,00	68,00	67,00	23,00	2	0	108,80	1 269,90	47 380,00	40,05	4 221,27
7	35,00	66,00	67,00	23,00	2	0	108,80	1 269,90	45 146,50	35,00	3 836,00
8	110,00	66,00	66,00	22,00	2	0	105,40	1 182,40	135 876,50	110,00	11 825,49
9	30,00	66,00	65,00	25,00	2	0	118,00	1 517,50	40 496,50	30,15	3 387,71
10	55,00	69,00	68,00	22,00	2	0	105,40	1 182,40	74 247,25	55,08	6 152,63
11	30,00	70,50	69,50	20,50	2	0	89,10	1 029,03	33 171,38	30,04	3 071,33
12	90,00	78,00	75,00	15,00	1	1	73,00	622,50	74 318,63	90,17	7 758,95
13	50,00	80,00	79,00	11,00	1	1	56,20	384,10	24 665,00	50,18	3 240,32
14	85,00	80,00	89,00	1,00	0	3	11,20	99,10	19 666,00	85,58	2 684,28

895,00  
 COMPR DO  
 EIXO

565 021,75  
 VOLUME DE  
 MACIÇO

55 984,29  
 ÁREA DE  
 FUNDAÇÃO

Exeno de De'

$$\textcircled{T} (0,5 \times 5) \times 570 = 1.425 \text{ m}^3$$

$$\textcircled{E} 20 \times 570 = 11.400 \text{ m}^3$$

$$\sqrt[3]{V_{\text{Comp}}} = 565.025 + 1.425 + 11.400 = 577.850 \text{ m}^3$$

ESTUDO DE OTIMIZAÇÃO  
ALTERNATIVA 3

COTA DA CRISTA - 30,00  
COTA DO NA - 55,00

$$V_{comp} = 580\,000 \text{ m}^3 \quad (-58\% \text{ de a}^{\text{a}} \text{ e } r^{\text{a}} \text{ e } c^{\text{a}})$$

- ⊖ 86,4%
- ⊖ 5,8%
- ⊖ 1,8%
- ⊖ 1,8%
- ⊖ 2,2%

Jo uncs - 0,1%

- Ⓢ 512 800 m<sup>3</sup>
- Ⓢ 33 700 m<sup>3</sup>
- ⊖ 10.400 m<sup>3</sup>
- Ⓢ 10 400 m<sup>3</sup>
- Ⓢ 12 800 m<sup>3</sup>

Desmatamento - 115 000 - 58% = 48 300 m<sup>2</sup>  
Trat em solo - 78 200 - 58% = 32 600 m<sup>2</sup>  
Trat. em rocha - 36.800 - 58% = 15 500 m<sup>2</sup>

Escavação

Comum - 168 600 - 58% = 70 800 m<sup>3</sup>  
Rocha - 6.000 m<sup>3</sup>

## Ensecadeiras (ALT 3)

1 - Ens 1ª Fase

$$MD - \left(84/2 \times 80\right) + (84 \times 370) = 34440$$

$$ME - \left(84/2 \times 210\right) + (84 \times 270) = 31500 \quad \left. \begin{array}{l} \\ \end{array} \right\} 65940$$

2 - Ens 2ª Fase

$$\text{Jus} - 9800$$

$$\text{Man} - 55300 \quad \left. \begin{array}{l} \\ \end{array} \right\} 65100$$

$$\text{Total} - 65940 + 65100 - 900^{\text{anch.}} = 125940$$

$$\textcircled{5} 146.000 \text{ m}^3$$

Remoção

$$MD - 84 \times 80 = 15120 \text{ m}^3$$

$$ME - (84 \times 280) + 5355 = 29875 \text{ m}^3 \quad \dots 44000$$